



Resistencia de Materiales

EL SÓLIDO ELÁSTICO (Ley de comportamiento)

CONTENIDO DE LA ASIGNATURA

BLOQUE TEMÁTICO: ELASTICIDAD Y RESISTENCIA DE MATERIALES

CAPÍTULO 1: INTRODUCCIÓN A LA ELASTICIDAD Y LA RESISTENCIA DE MATERIALES

CAPÍTULO 2: EL SÓLIDO ELÁSTICO.

CAPÍTULO 3: CRITERIOS DE PLASTIFICACIÓN Y DE ROTURA

CAPÍTULO 4: RESISTENCIA DE MATERIALES. CONCEPTOS BÁSICOS

CAPÍTULO 5: TRACCIÓN Y COMPRESIÓN

CAPÍTULO 6: FLEXIÓN PLANA ELÁSTICA.

CAPÍTULO 7: INTRODUCCIÓN AL CÁLCULO PLÁSTICO

CAPÍTULO 8: FLEXO-COMPRESIÓN DESVIADA

CAPÍTULO 9: TORSIÓN

CAPÍTULO 10: POTENCIAL ELÁSTICO DE BARRAS. MÉTODOS ENERGÉTICOS

CAPÍTULO 11: INESTABILIDAD DE BARRAS PRISMÁTICAS. PANDEO

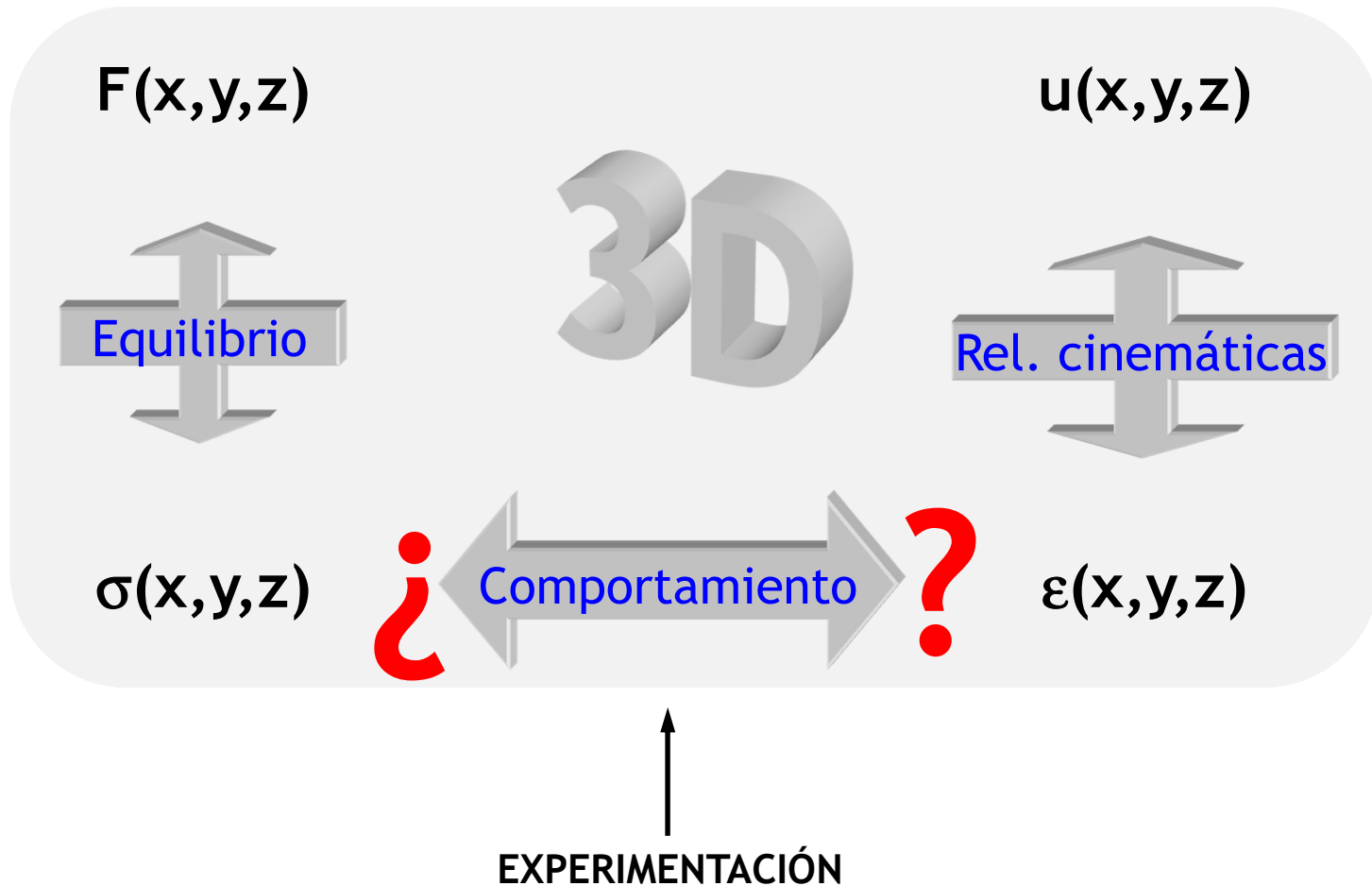


Resistencia de Materiales

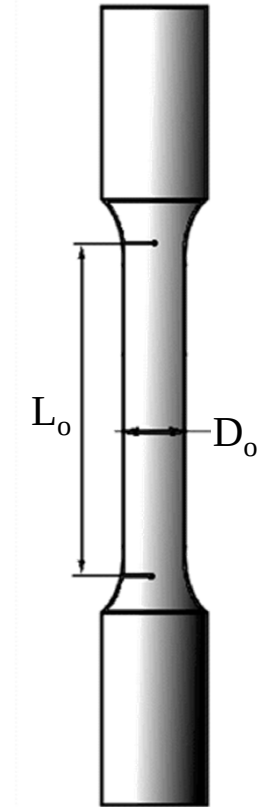
EL SÓLIDO ELÁSTICO (Ley de comportamiento)

- Introducción.
- El ensayo de tracción monoaxial.
- Ley de Hooke generalizada.
- Módulo de cizalladura.
- Ley de comportamiento en unas coordenadas cualesquiera.
- El problema elástico.
- Condiciones de contorno.
- El problema térmico.
- Energía de deformación.
- Principio de Saint-Venant.

El problema elástico



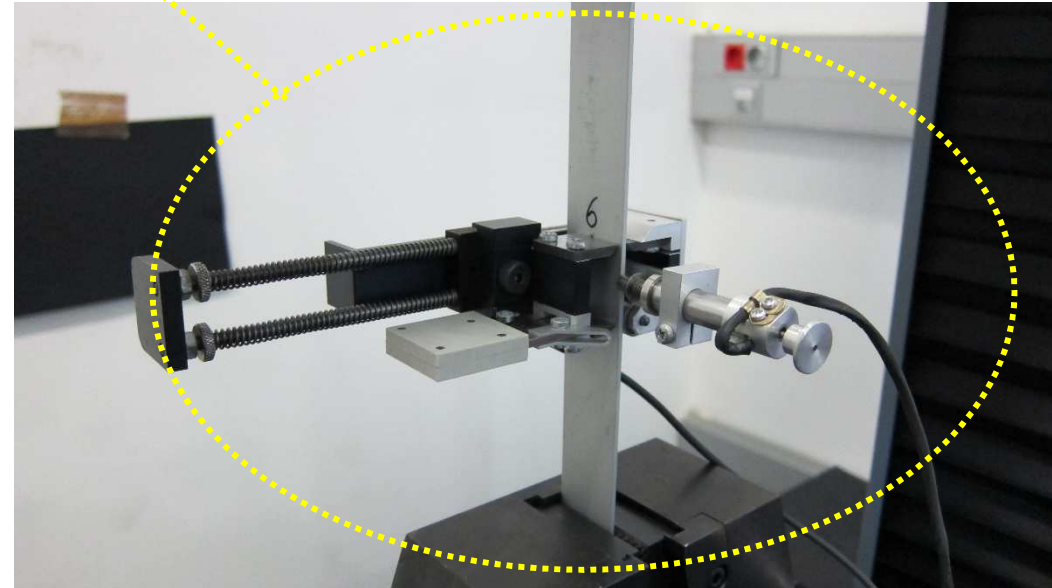
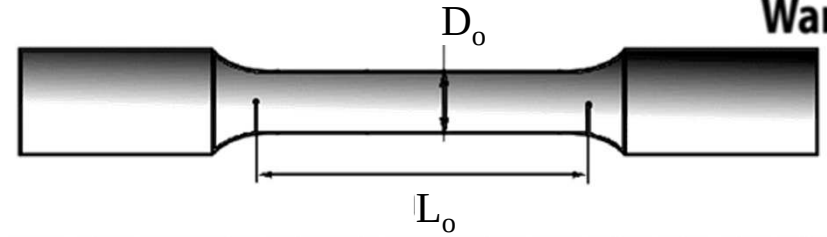
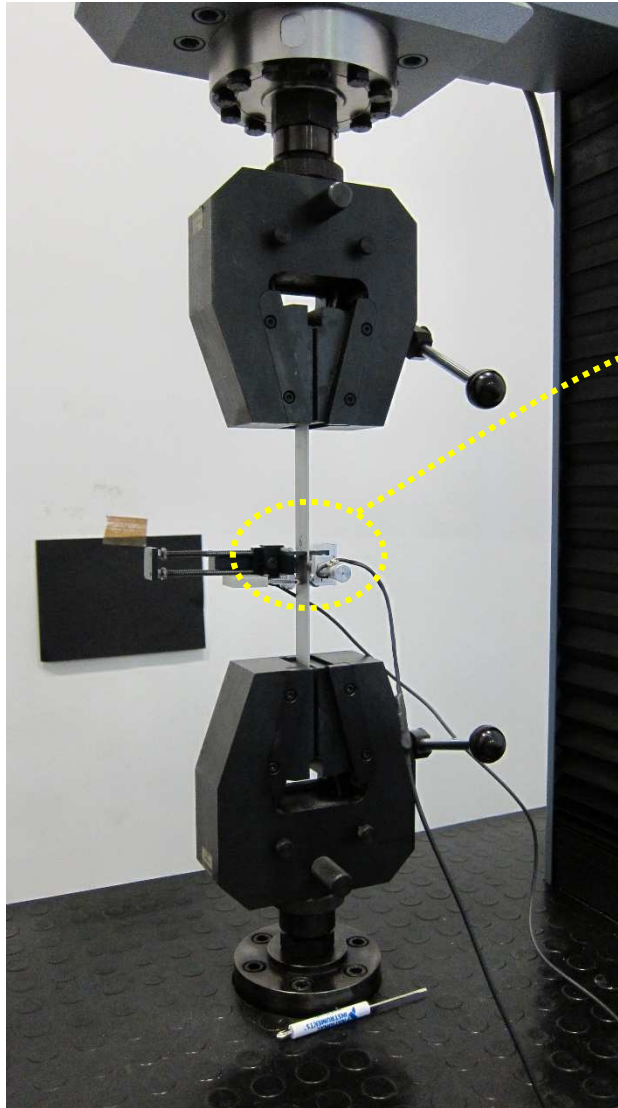
Ensayo de tracción



* probeta cilíndrica tipo – recreación gráfica

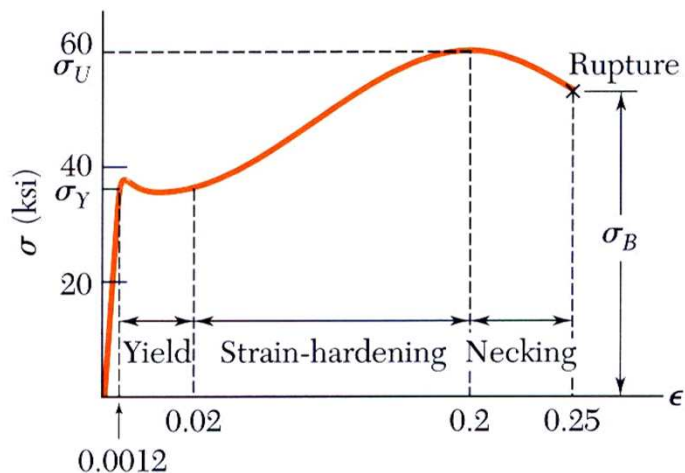
* máquina de ensayos del Laboratorio de Resistencia de Materiales - UMA

Ensayo de tracción

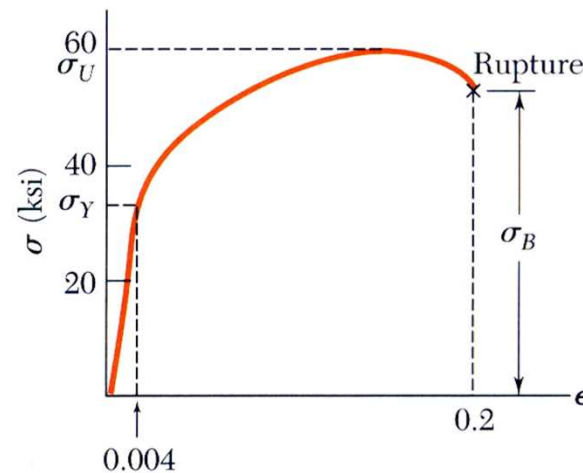


* Vista detallada del extensómetro electrónico fijado a la probeta

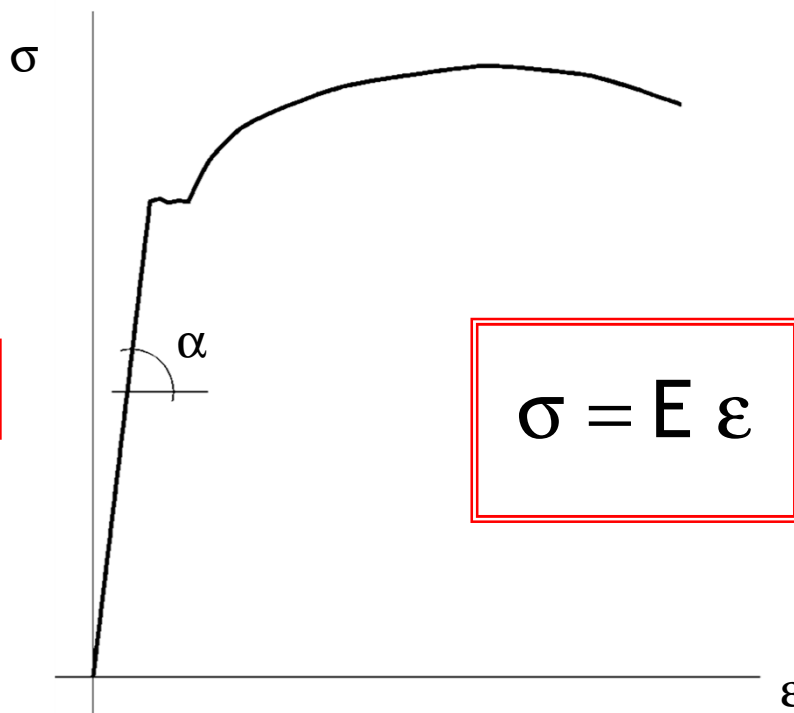
Ensayo de tracción



(a) Low-carbon steel



(b) Aluminum alloy



$$\text{tg}(\alpha) = E > 0$$

$$\sigma = E \epsilon$$

Thomas Young
13/06/1773 (Somerset, Inglaterra)-
-10/05/1829 (Londres)



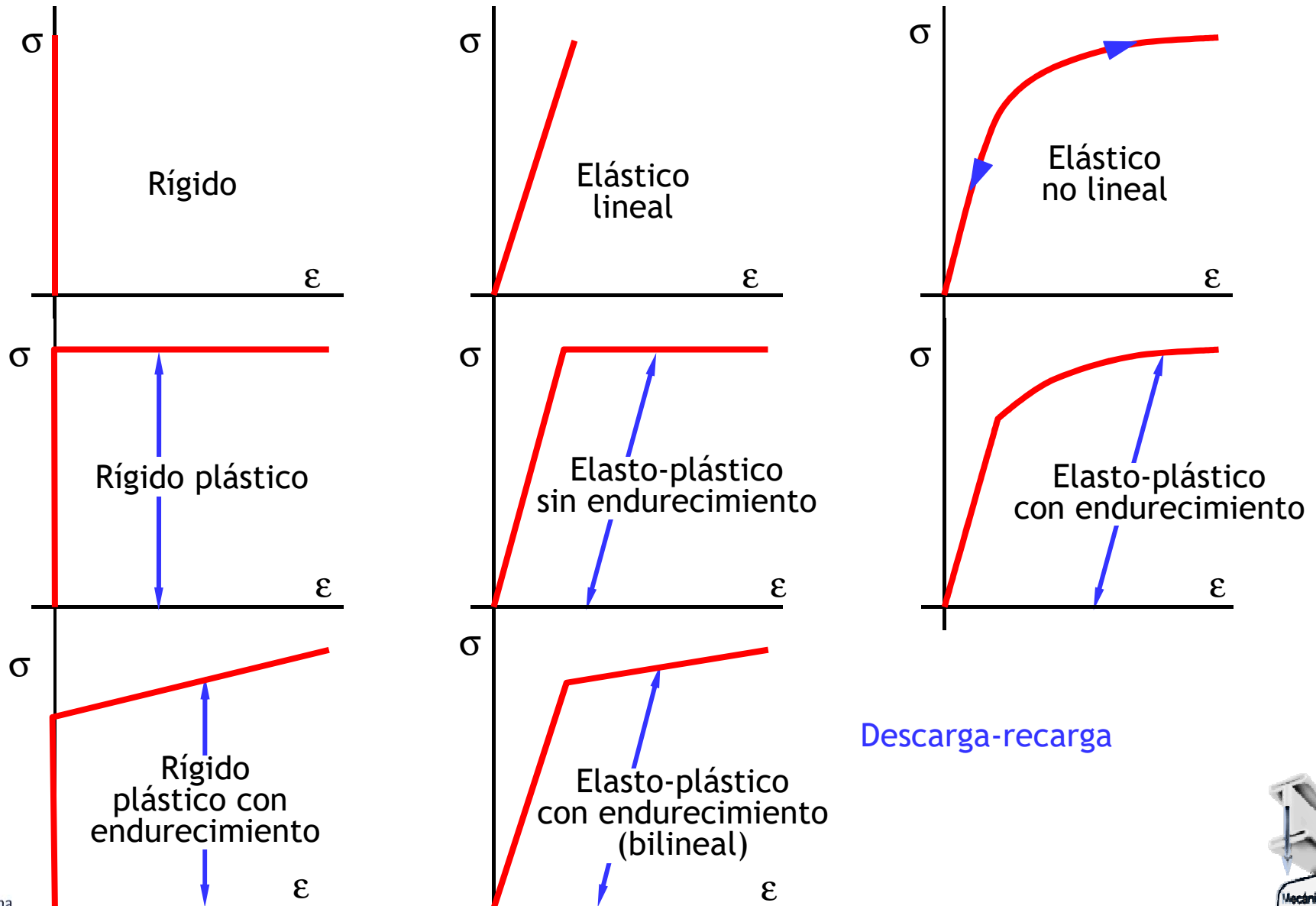
* fuente: wikipedia



Mecánica de
Medios Continuos y
Teoría de Estructuras

Ensayo de tracción

Diferentes modelos de comportamiento



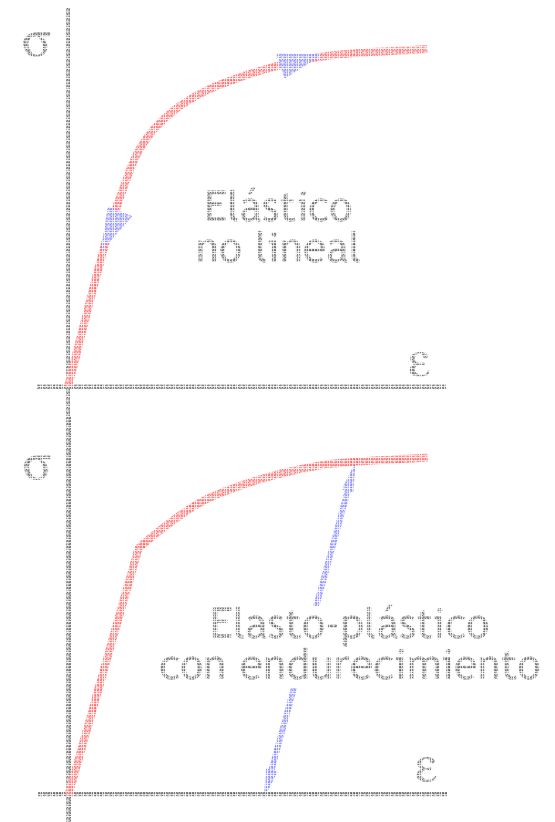
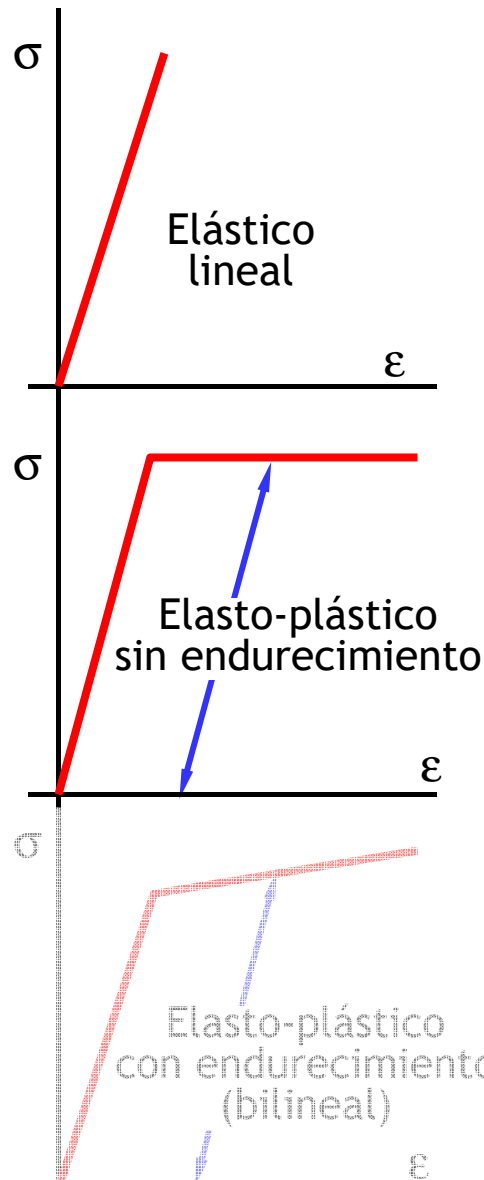
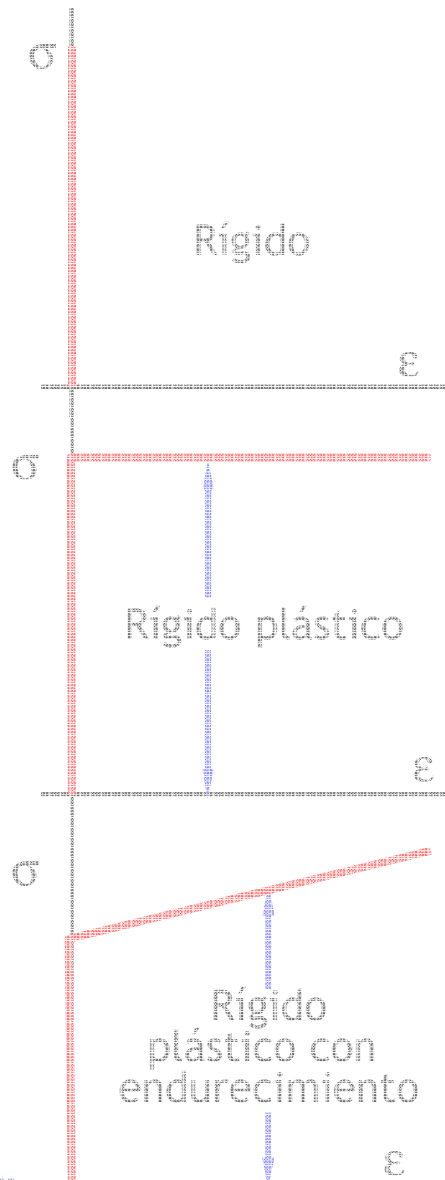
Descarga-recarga



Mecánica de
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Ensayo de tracción

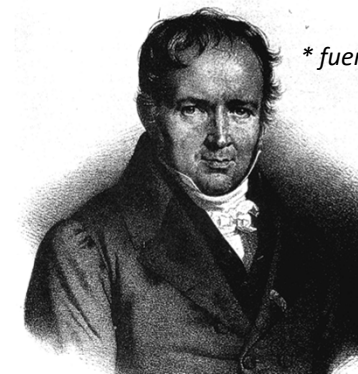
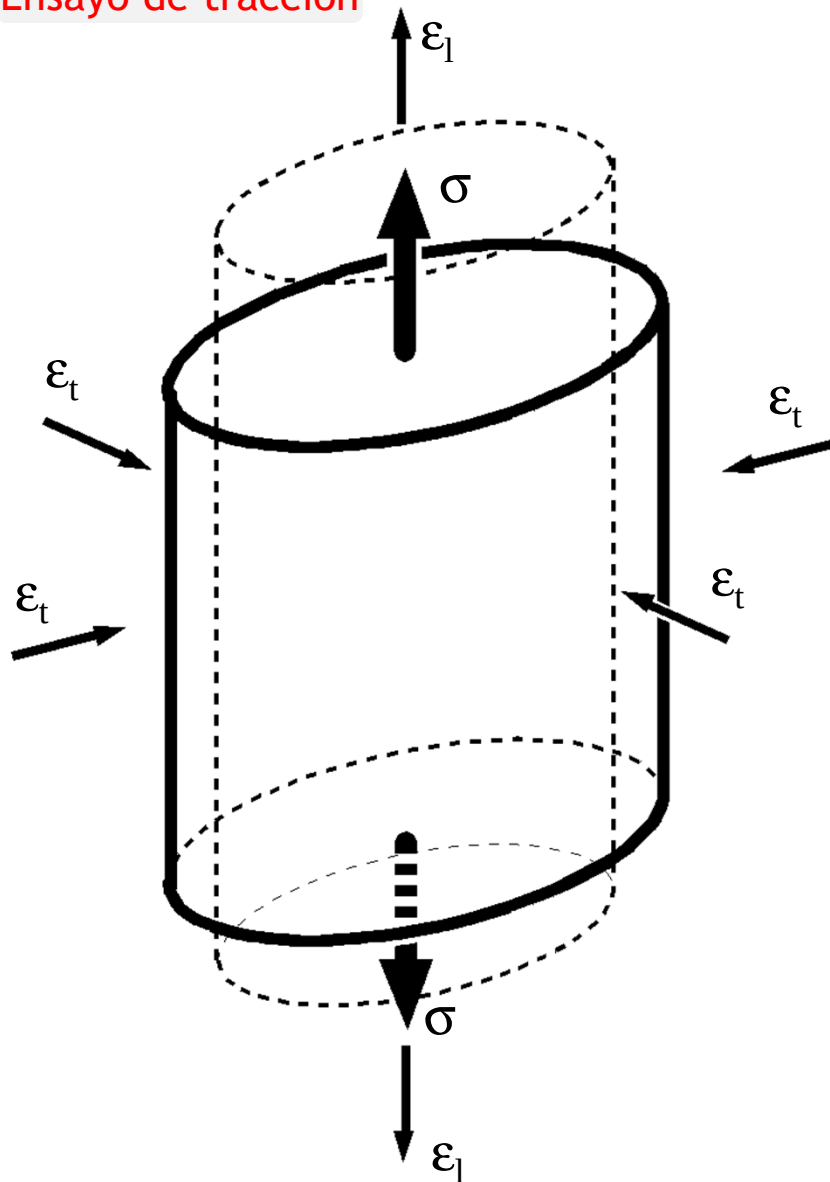
Diferentes modelos de comportamiento



Descarga-recarga



Ensayo de tracción



* fuente: wikipedia

Simeón Denis Poisson
 21/06/1781 (Pithiviers, Francia)-
 -25/04/1840 (Sceaux, Francia)

$$\varepsilon_t = -\nu \varepsilon_l$$

Nota: $\nu > 0$

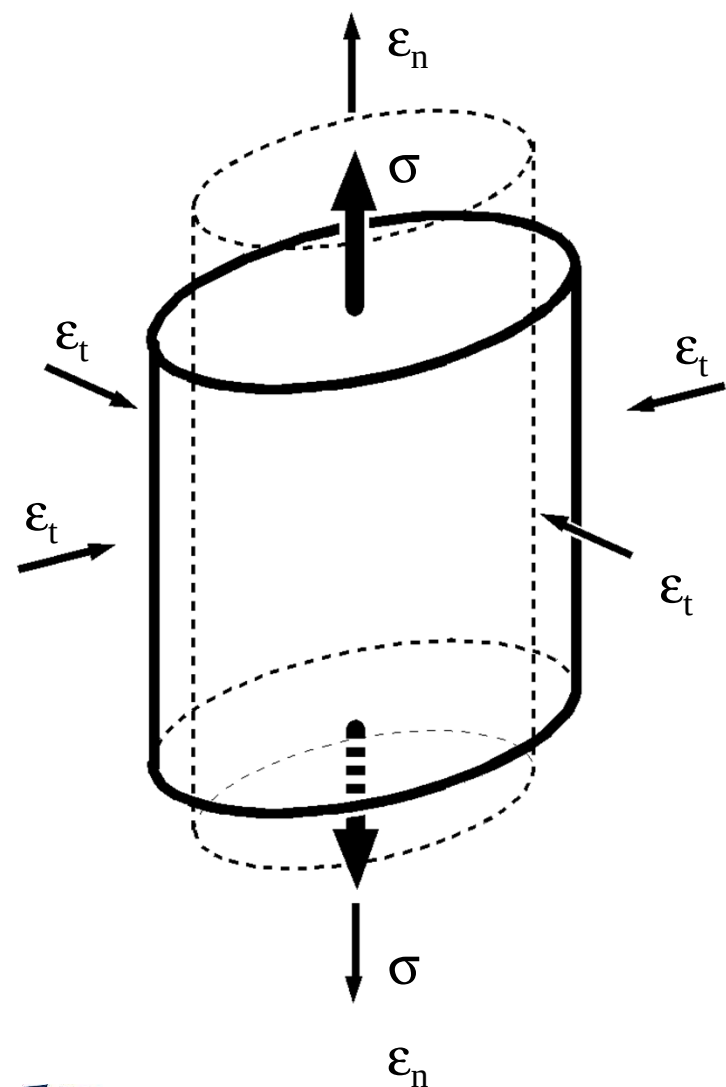


Ley de Hooke generalizada

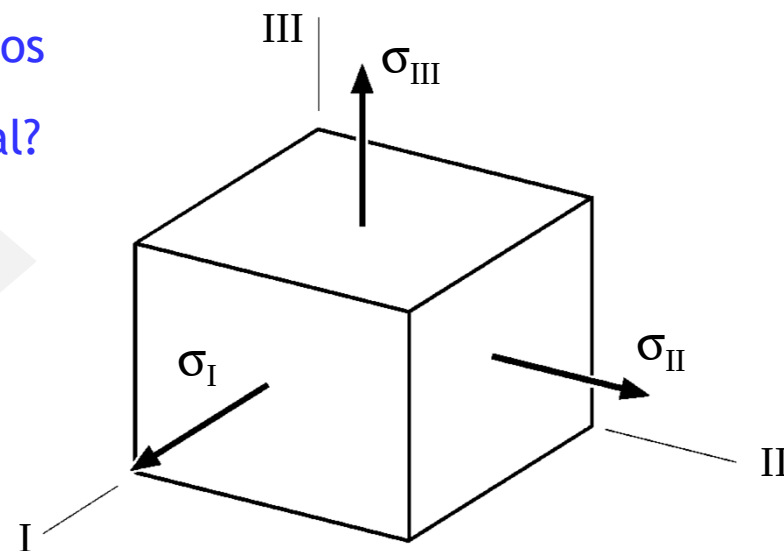
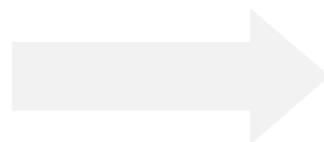


* fuente: wikipedia

Robert Hooke
18/07/1635 (Freshwater, Inglaterra)-
-03/03/1703 (Londres)



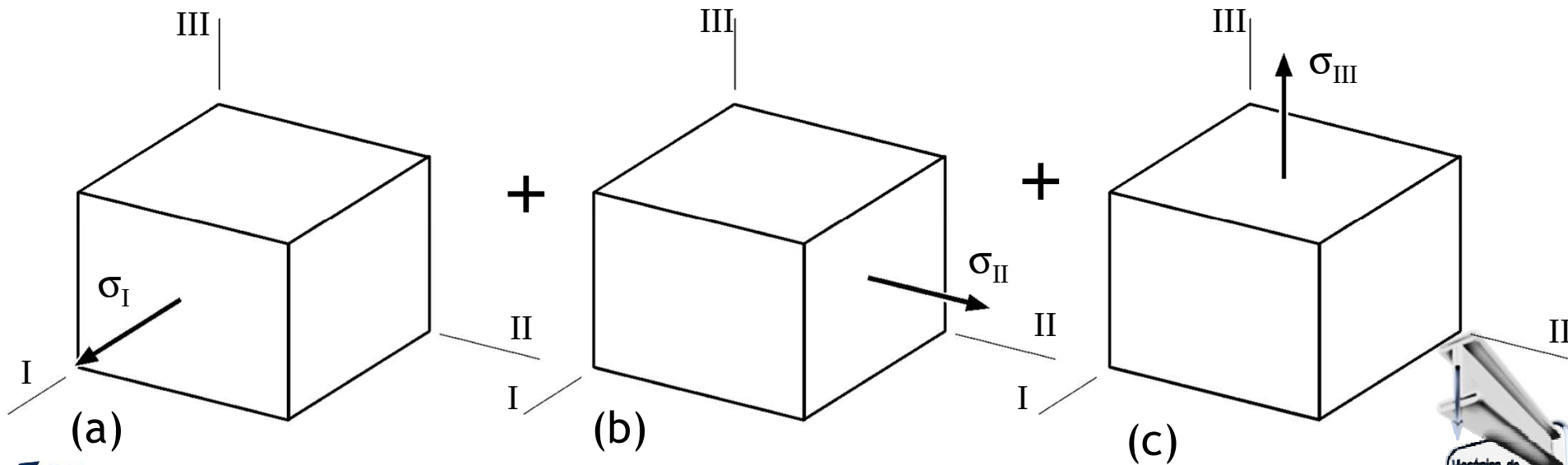
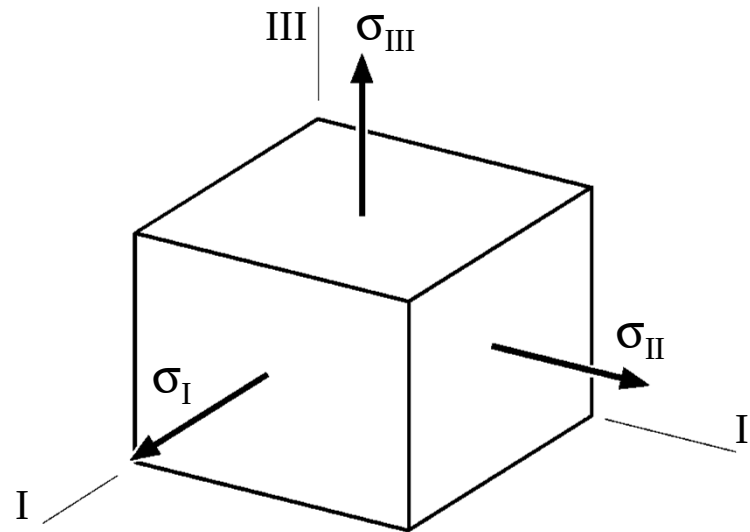
¿Cómo pasamos
al caso
tridimensional?



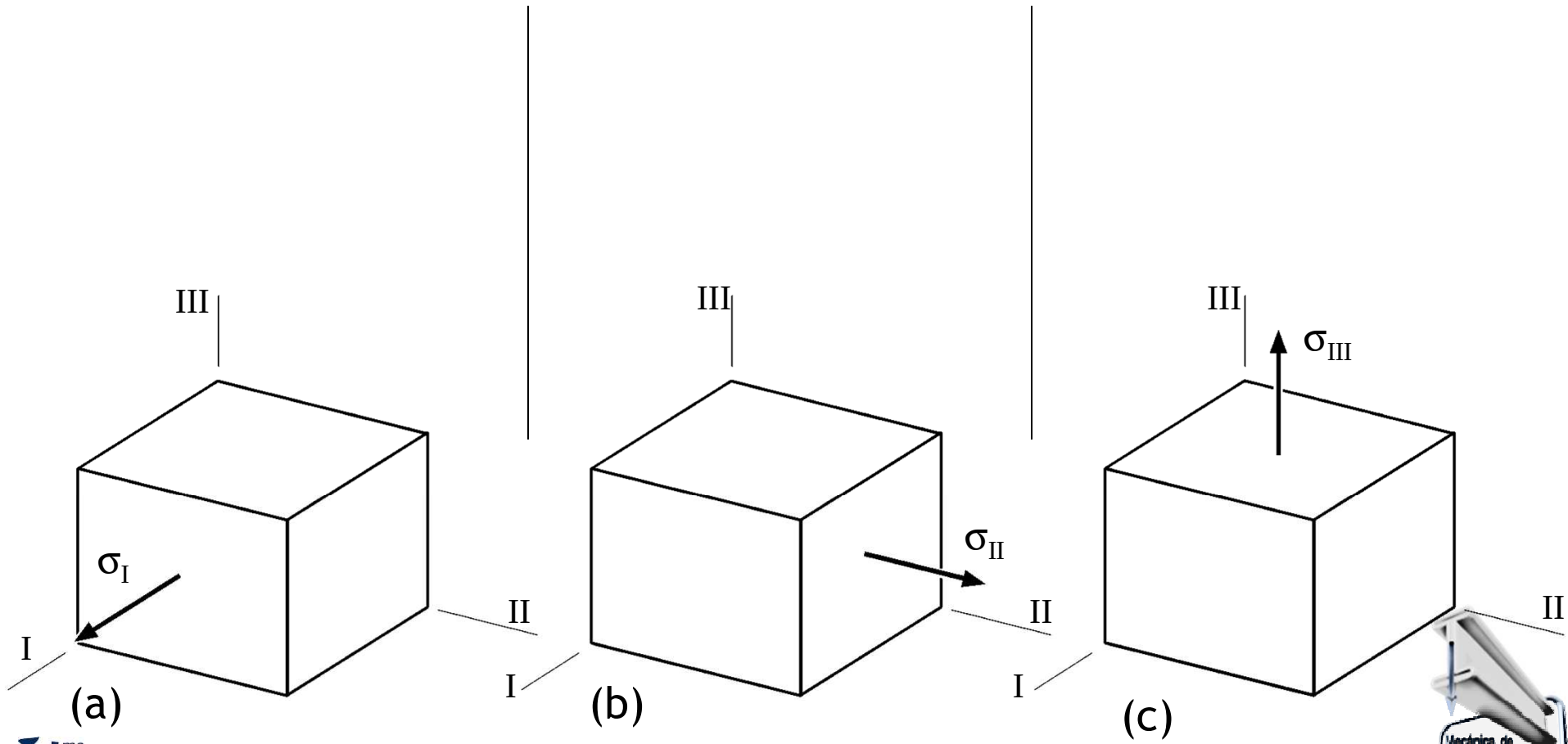
Mecánica de
Medios Continuos y
Teoría de Estructuras

Ley de Hooke generalizada

Principio de superposición



Ley de Hooke generalizada

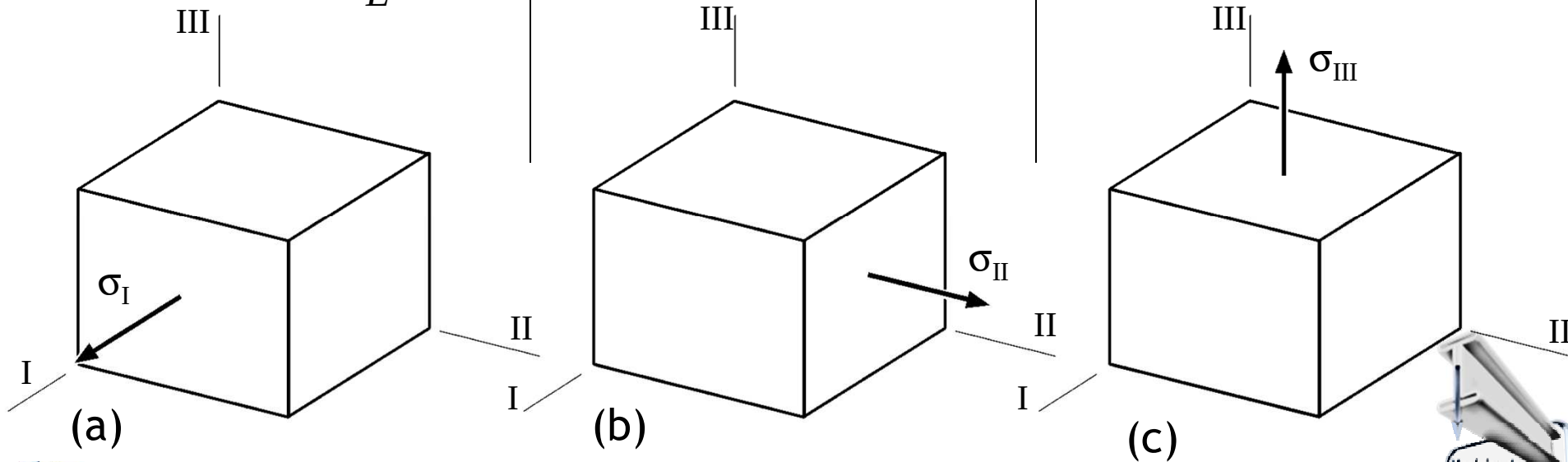


Ley de Hooke generalizada

$$\varepsilon_I = \frac{\sigma_I}{E}$$

$$\varepsilon_{II} = -\nu \varepsilon_I = -\nu \frac{\sigma_I}{E}$$

$$\varepsilon_{III} = -\nu \varepsilon_I = -\nu \frac{\sigma_I}{E}$$

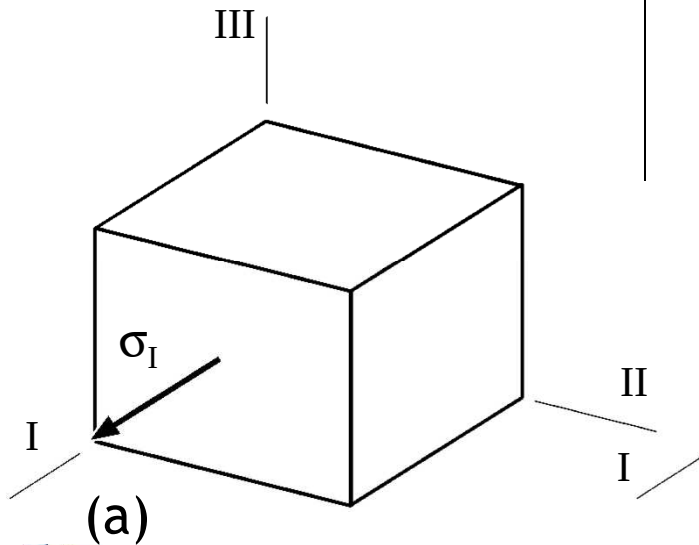


Ley de Hooke generalizada

$$\varepsilon_I = \frac{\sigma_I}{E}$$

$$\varepsilon_{II} = -\nu \varepsilon_I = -\nu \frac{\sigma_I}{E}$$

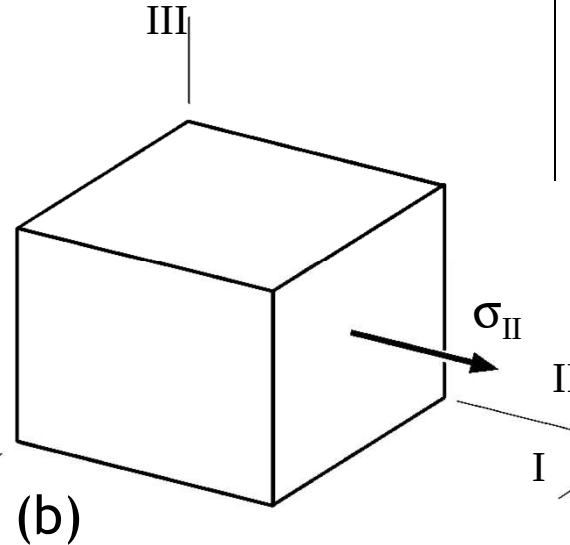
$$\varepsilon_{III} = -\nu \varepsilon_I = -\nu \frac{\sigma_I}{E}$$



$$\varepsilon_I = -\nu \varepsilon_{II} = -\nu \frac{\sigma_{II}}{E}$$

$$\varepsilon_{II} = \frac{\sigma_{II}}{E}$$

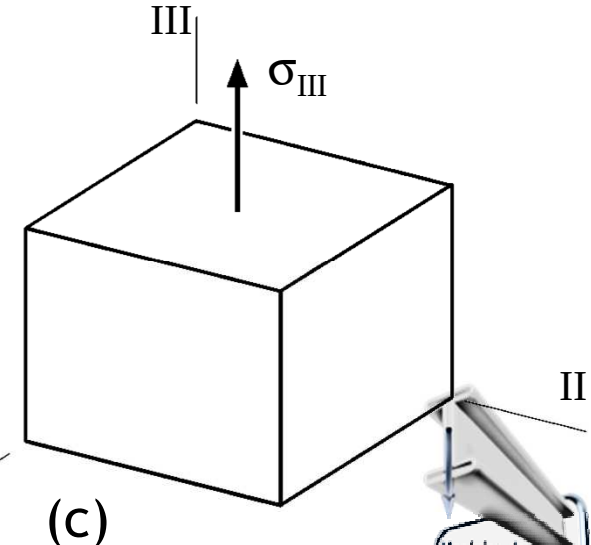
$$\varepsilon_{III} = -\nu \varepsilon_{II} = -\nu \frac{\sigma_{II}}{E}$$



$$\varepsilon_I = -\nu \varepsilon_{III} = -\nu \frac{\sigma_{III}}{E}$$

$$\varepsilon_{II} = -\nu \varepsilon_{III} = -\nu \frac{\sigma_{III}}{E}$$

$$\varepsilon_{III} = \frac{\sigma_{III}}{E}$$

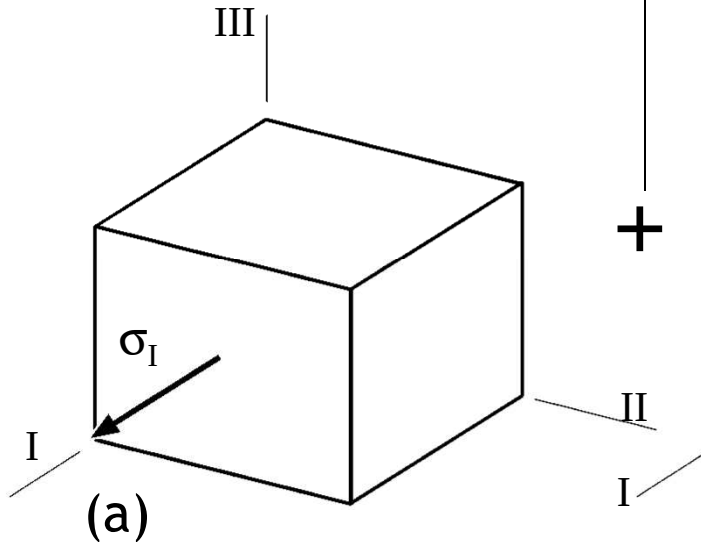


Ley de Hooke generalizada

$$\varepsilon_I = \frac{\sigma_I}{E}$$

$$\varepsilon_{II} = -\nu \varepsilon_I = -\nu \frac{\sigma_I}{E}$$

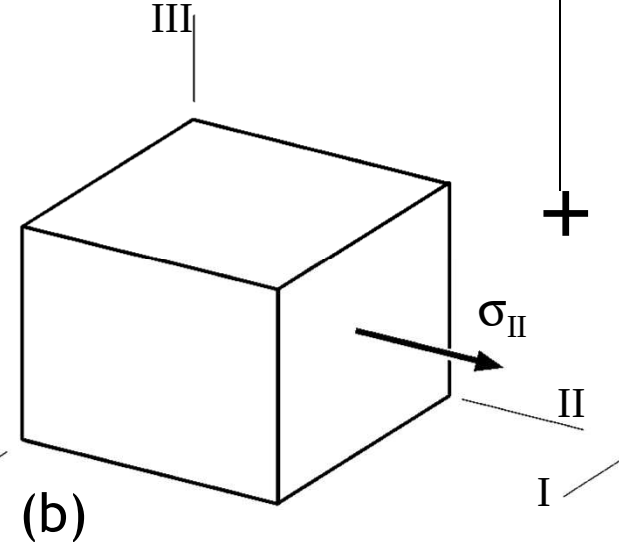
$$\varepsilon_{III} = -\nu \varepsilon_I = -\nu \frac{\sigma_I}{E}$$



$$\varepsilon_I = -\nu \varepsilon_{II} = -\nu \frac{\sigma_{II}}{E}$$

$$\varepsilon_{II} = \frac{\sigma_{II}}{E}$$

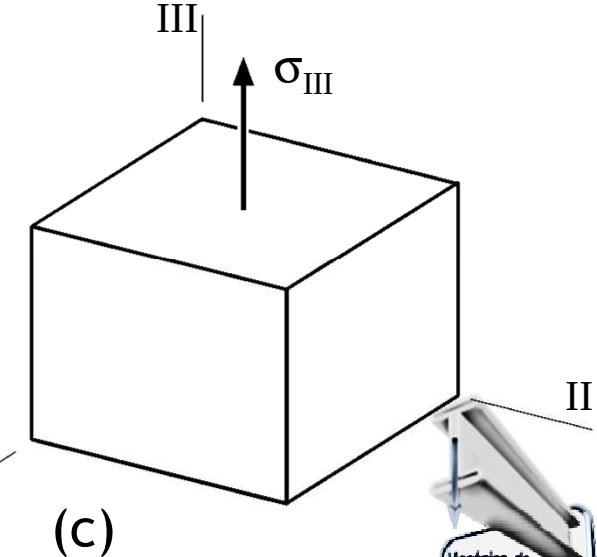
$$\varepsilon_{III} = -\nu \varepsilon_{II} = -\nu \frac{\sigma_{II}}{E}$$



$$\varepsilon_I = -\nu \varepsilon_{III} = -\nu \frac{\sigma_{III}}{E}$$

$$\varepsilon_{II} = -\nu \varepsilon_{III} = -\nu \frac{\sigma_{III}}{E}$$

$$\varepsilon_{III} = \frac{\sigma_{III}}{E}$$



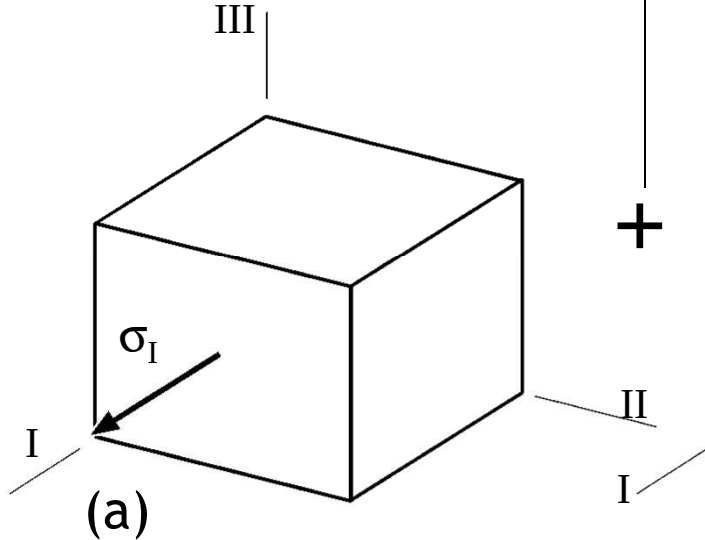
Ley de Hooke generalizada

$$\varepsilon_I = \frac{\sigma_I - \nu(\sigma_{II} + \sigma_{III})}{E}$$

$$\varepsilon_I = \frac{\sigma_I}{E}$$

$$\varepsilon_{II} = -\nu\varepsilon_I = -\nu\frac{\sigma_I}{E}$$

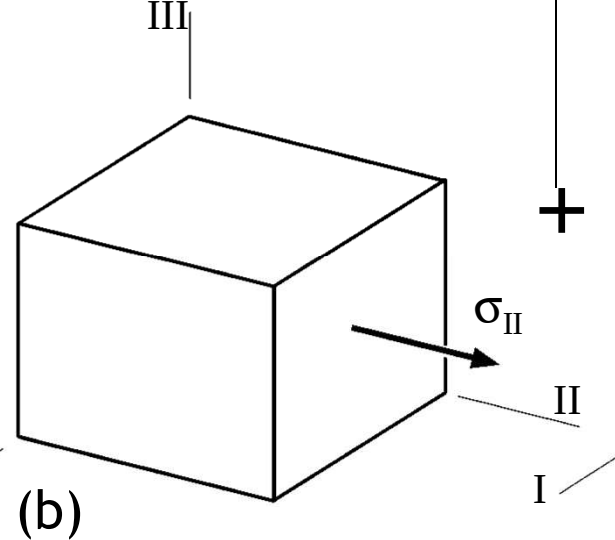
$$\varepsilon_{III} = -\nu\varepsilon_I = -\nu\frac{\sigma_I}{E}$$



$$\varepsilon_I = -\nu\varepsilon_{II} = -\nu\frac{\sigma_{II}}{E}$$

$$\varepsilon_{II} = \frac{\sigma_{II}}{E}$$

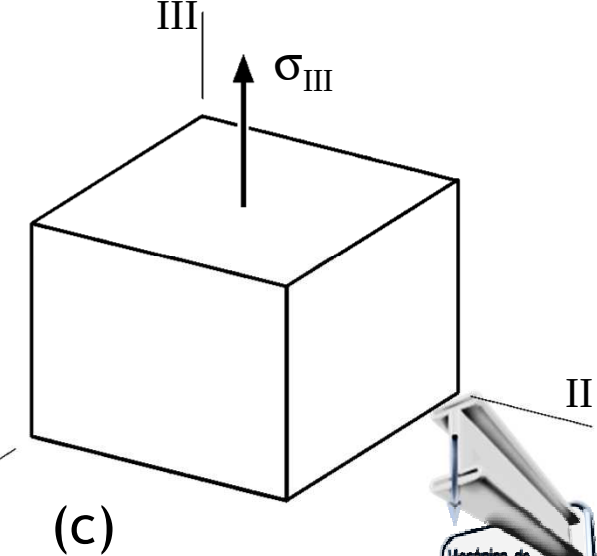
$$\varepsilon_{III} = -\nu\varepsilon_{II} = -\nu\frac{\sigma_{II}}{E}$$



$$\varepsilon_I = -\nu\varepsilon_{III} = -\nu\frac{\sigma_{III}}{E}$$

$$\varepsilon_{II} = -\nu\varepsilon_{III} = -\nu\frac{\sigma_{III}}{E}$$

$$\varepsilon_{III} = \frac{\sigma_{III}}{E}$$



Ley de Hooke generalizada

$$\varepsilon_I = \frac{\sigma_I - \nu(\sigma_{II} + \sigma_{III})}{E}$$

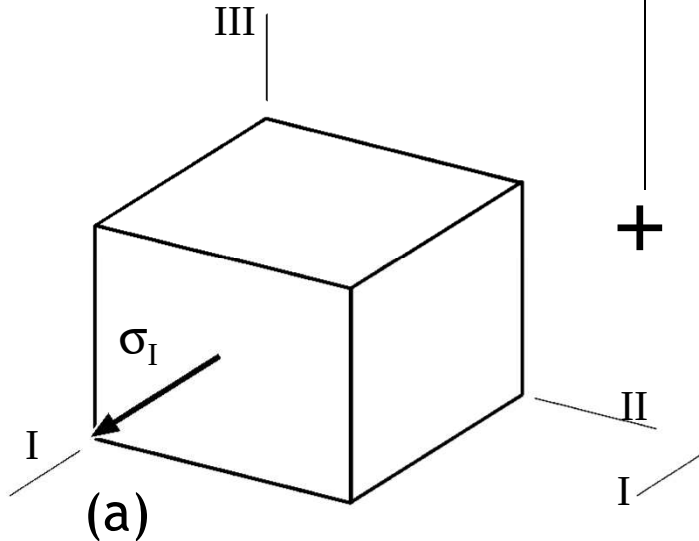
$$\varepsilon_{II} = \frac{\sigma_{II} - \nu(\sigma_I + \sigma_{III})}{E}$$

$$\varepsilon_{III} = \frac{\sigma_{III} - \nu(\sigma_I + \sigma_{II})}{E}$$

$$\varepsilon_I = \frac{\sigma_I}{E}$$

$$\varepsilon_{II} = -\nu\varepsilon_I = -\nu\frac{\sigma_I}{E}$$

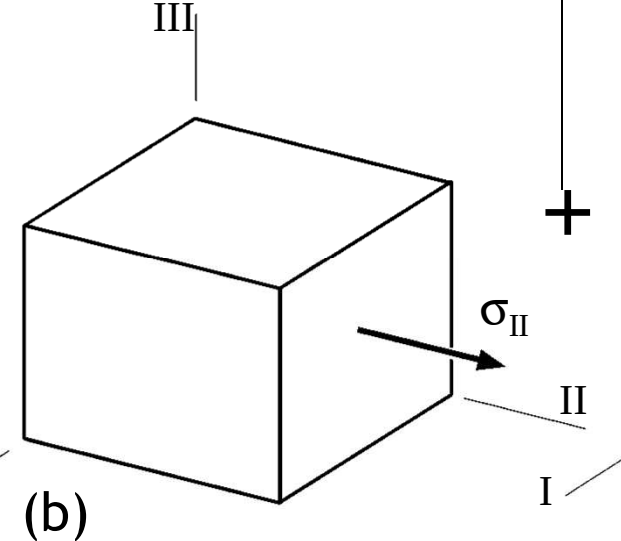
$$\varepsilon_{III} = -\nu\varepsilon_I = -\nu\frac{\sigma_I}{E}$$



$$\varepsilon_I = -\nu\varepsilon_{II} = -\nu\frac{\sigma_{II}}{E}$$

$$\varepsilon_{II} = \frac{\sigma_{II}}{E}$$

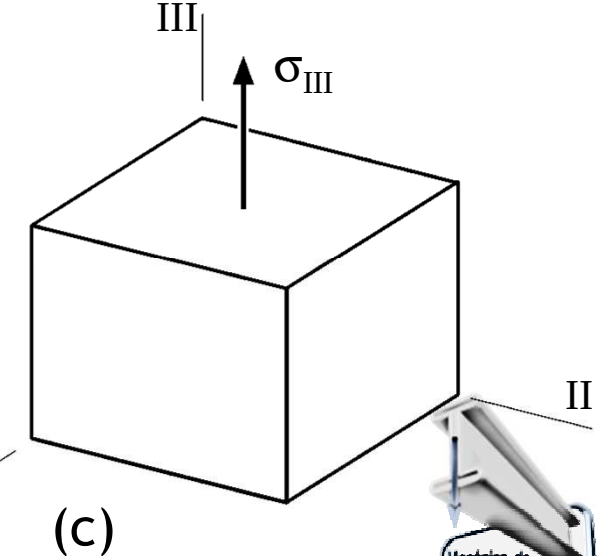
$$\varepsilon_{III} = -\nu\varepsilon_{II} = -\nu\frac{\sigma_{II}}{E}$$



$$\varepsilon_I = -\nu\varepsilon_{III} = -\nu\frac{\sigma_{III}}{E}$$

$$\varepsilon_{II} = -\nu\varepsilon_{III} = -\nu\frac{\sigma_{III}}{E}$$

$$\varepsilon_{III} = \frac{\sigma_{III}}{E}$$



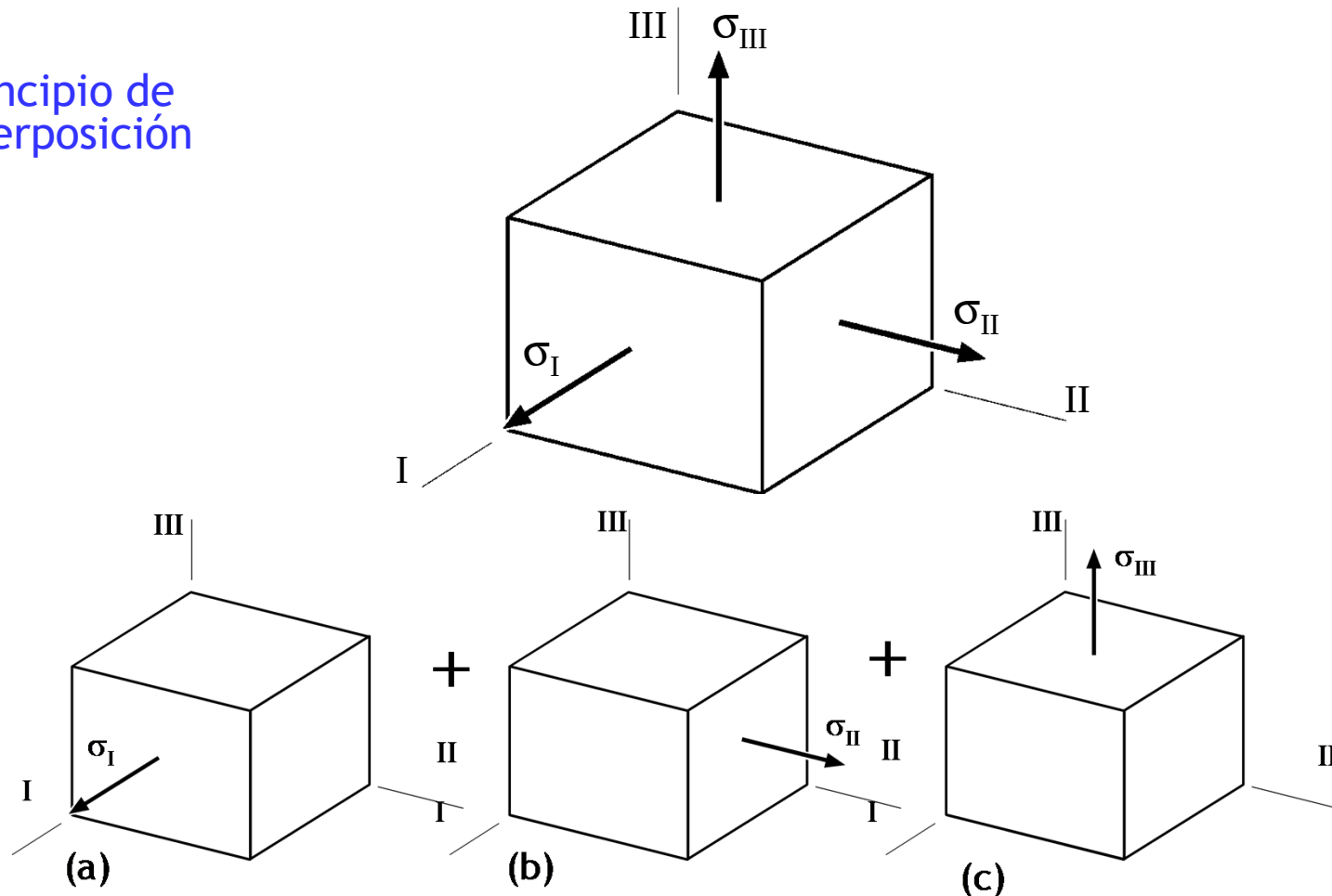
Ley de Hooke generalizada

$$\varepsilon_I = \frac{\sigma_I - \nu(\sigma_{II} + \sigma_{III})}{E}$$

$$\varepsilon_{II} = \frac{\sigma_{II} - \nu(\sigma_I + \sigma_{III})}{E}$$

$$\varepsilon_{III} = \frac{\sigma_{III} - \nu(\sigma_I + \sigma_{II})}{E}$$

Principio de superposición



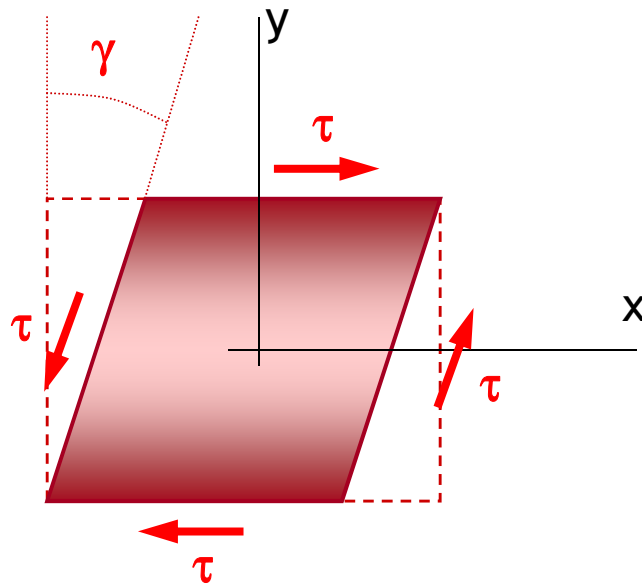
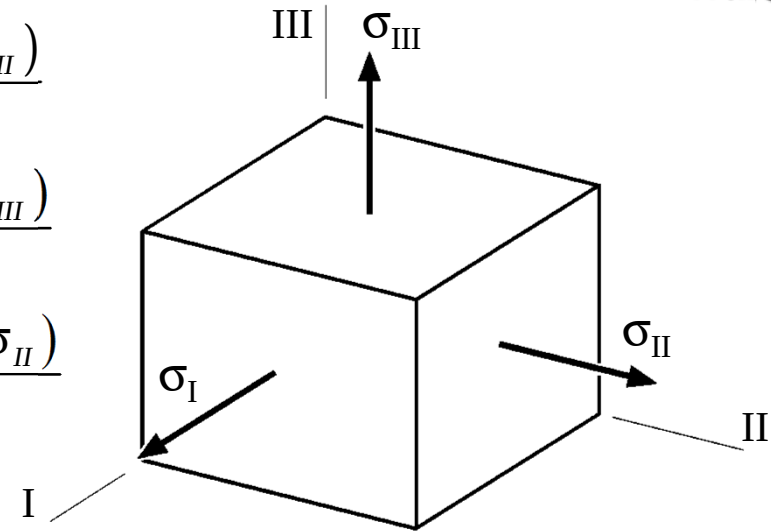
Módulo de cizalladura

Relaciones
tensión/deformación
normales

$$\varepsilon_I = \frac{\sigma_I - \nu(\sigma_{II} + \sigma_{III})}{E}$$

$$\varepsilon_{II} = \frac{\sigma_{II} - \nu(\sigma_I + \sigma_{III})}{E}$$

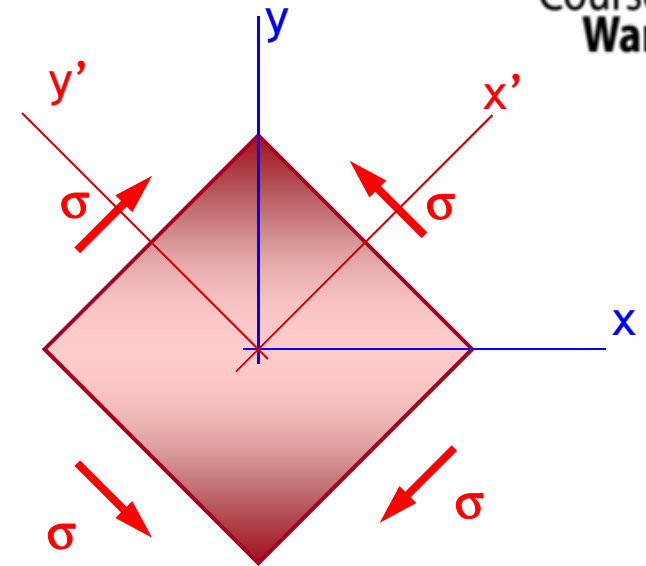
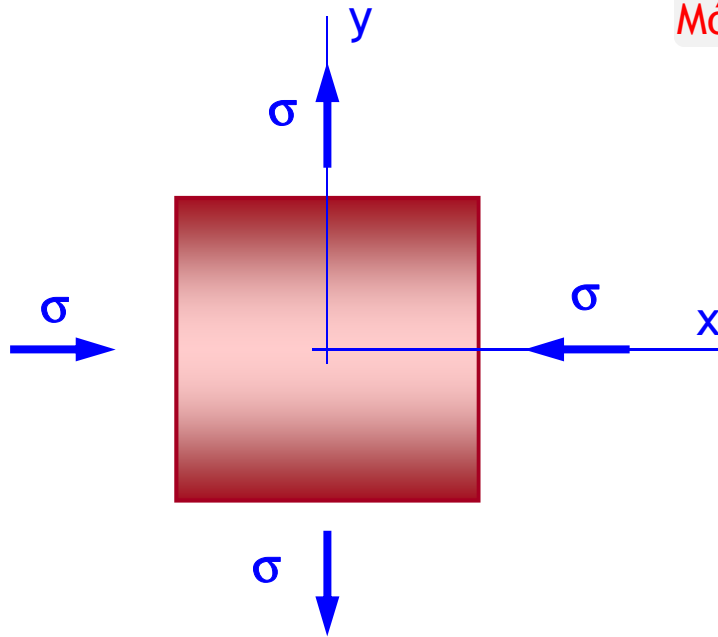
$$\varepsilon_{III} = \frac{\sigma_{III} - \nu(\sigma_I + \sigma_{II})}{E}$$



¿Qué ocurre con las
componentes tangenciales
y las
deformaciones angulares?



Módulo de cizalladura



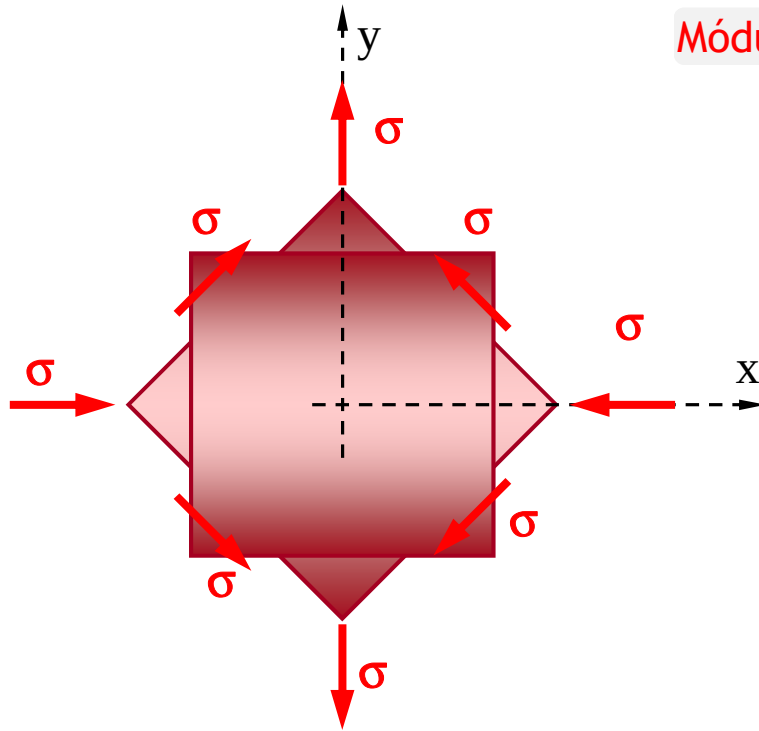
MISMO
estado tensional
utilizando un
sistema de referencia
diferente

$$\sigma = \begin{pmatrix} -\sigma & 0 & 0 \\ 0 & \sigma & 0 \\ 0 & 0 & 0 \end{pmatrix}$$

$$\sigma' = \begin{pmatrix} 0 & \sigma & 0 \\ \sigma & 0 & 0 \\ 0 & 0 & 0 \end{pmatrix}$$



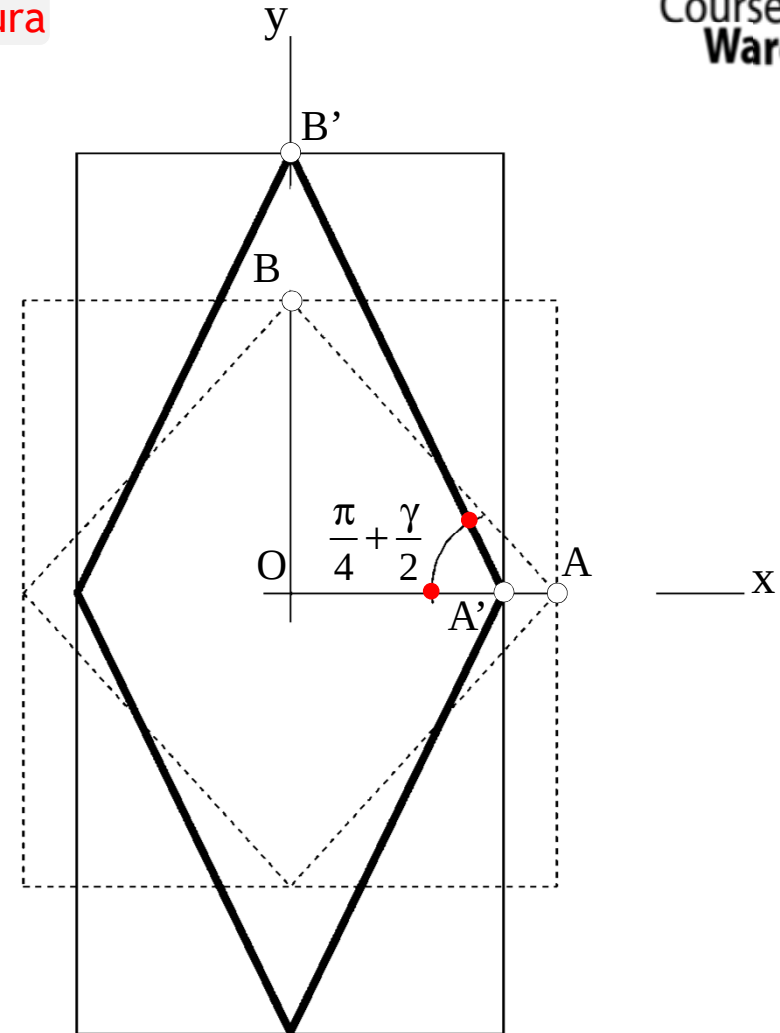
Módulo de cizalladura



$$\varepsilon_x = \frac{-\sigma - \nu \sigma}{E} = -(1 + \nu) \frac{\sigma}{E}$$

$$\varepsilon_y = \frac{\sigma - \nu(-\sigma)}{E} = (1 + \nu) \frac{\sigma}{E}$$

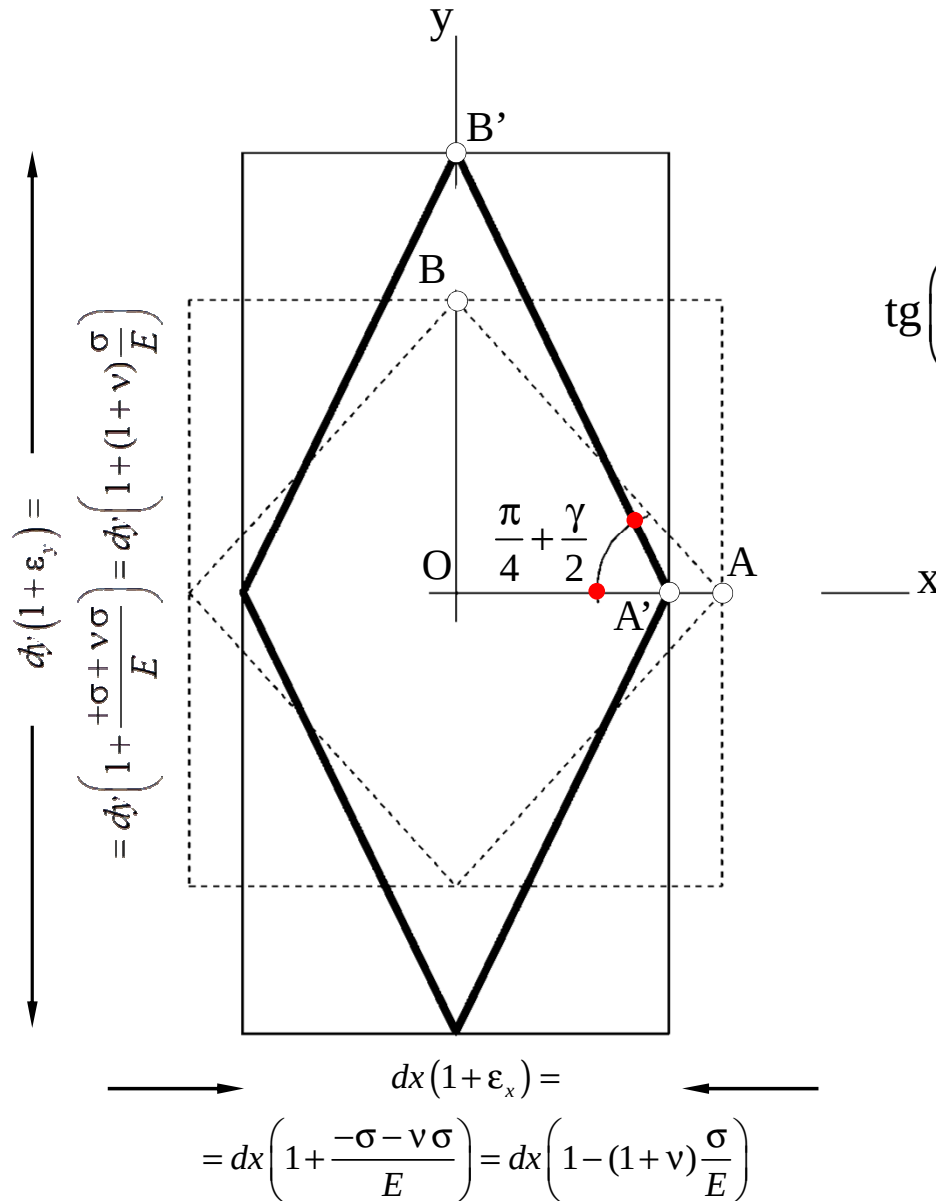
$$dy(1 + \varepsilon_y) = dy \left(1 + \frac{\sigma + \nu \sigma}{E} \right) = dy \left(1 + (1 + \nu) \frac{\sigma}{E} \right)$$



$$dx(1 + \varepsilon_x) = dx \left(1 + \frac{-\sigma - \nu \sigma}{E} \right) = dx \left(1 - (1 + \nu) \frac{\sigma}{E} \right)$$



Módulo de cizalladura



$$\operatorname{tg} \left(\frac{\pi}{4} + \frac{\gamma}{2} \right) = \frac{1 + \epsilon_y}{1 + \epsilon_x} = \frac{1 + (1 + \nu) \sigma / E}{1 - (1 + \nu) \sigma / E}$$

$$\operatorname{tg} \left(\frac{\pi}{4} + \frac{\gamma}{2} \right) = \frac{\operatorname{tg}(\pi/4) + \operatorname{tg}(\gamma/2)}{1 - \operatorname{tg}(\pi/4) \operatorname{tg}(\gamma/2)} \approx \frac{1 + \gamma/2}{1 - \gamma/2}$$

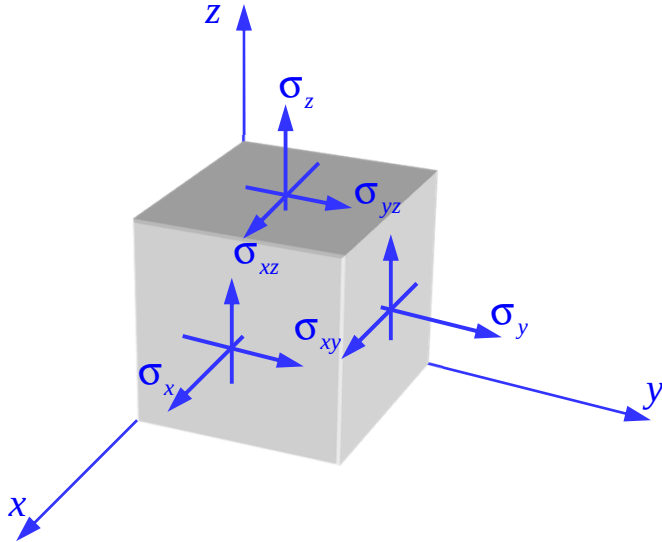
$$\frac{\gamma}{2} = \frac{(1 + \nu)}{E} \sigma := \frac{\sigma}{2G}$$

$$G = \frac{E}{2(1 + \nu)}$$

Nota: $G > 0$



Ley de comportamiento en coordenadas cualesquiera



$$\epsilon_x = \frac{\sigma_x - \nu(\sigma_y + \sigma_z)}{E} \quad (1)$$

$$\epsilon_y = \frac{\sigma_y - \nu(\sigma_x + \sigma_z)}{E} \quad (2)$$

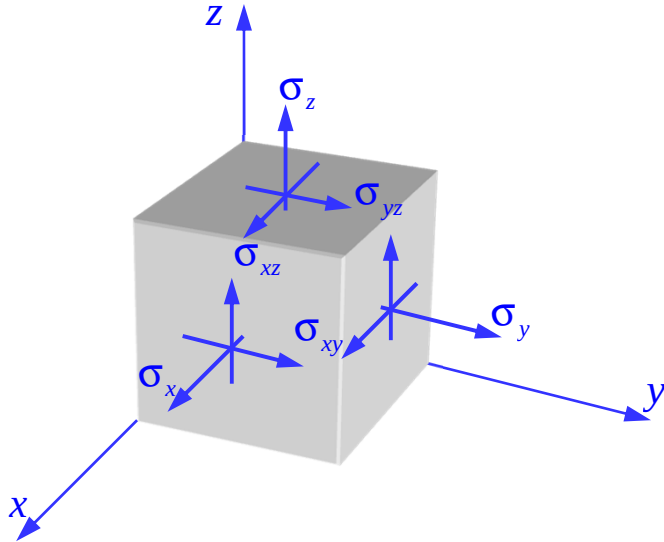
$$\epsilon_z = \frac{\sigma_z - \nu(\sigma_x + \sigma_y)}{E} \quad (3)$$

$$\epsilon_{xy} = \frac{(1+\nu)}{E} \sigma_{xy} \quad (4)$$

$$\epsilon_{xz} = \frac{(1+\nu)}{E} \sigma_{xz} \quad (5)$$

$$\epsilon_{yz} = \frac{(1+\nu)}{E} \sigma_{yz} \quad (7)$$

Ley de comportamiento en coordenadas cualesquiera



$$\epsilon_x = \frac{\sigma_x - \nu(\sigma_y + \sigma_z)}{E} \quad (1)$$

$$\epsilon_y = \frac{\sigma_y - \nu(\sigma_x + \sigma_z)}{E} \quad (2)$$

$$\epsilon_z = \frac{\sigma_z - \nu(\sigma_x + \sigma_y)}{E} \quad (3)$$

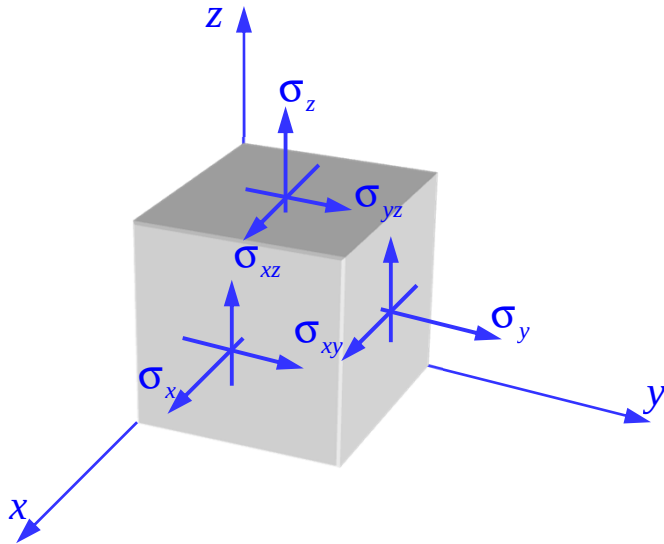
$$\epsilon_{xy} = \frac{(1+\nu)}{E} \sigma_{xy} \quad (4)$$

$$\epsilon_{xz} = \frac{(1+\nu)}{E} \sigma_{xz} \quad (5)$$

$$\epsilon_{yz} = \frac{(1+\nu)}{E} \sigma_{yz} \quad (7)$$

Dirección principal = Dirección de deformación = Dirección principal de tensión

Ley de comportamiento en coordenadas cualesquiera



$$\epsilon_x = \frac{\sigma_x - \nu(\sigma_y + \sigma_z)}{E} \quad (1)$$

$$\epsilon_y = \frac{\sigma_y - \nu(\sigma_x + \sigma_z)}{E} \quad (2)$$

$$\epsilon_z = \frac{\sigma_z - \nu(\sigma_x + \sigma_y)}{E} \quad (3)$$

$$\epsilon_{xy} = \frac{(1+\nu)}{E} \sigma_{xy} \quad (4)$$

$$\epsilon_{xz} = \frac{(1+\nu)}{E} \sigma_{xz} \quad (5)$$

$$\epsilon_{yz} = \frac{(1+\nu)}{E} \sigma_{yz} \quad (7)$$

(1)+(2)+(3)

$$\epsilon_x + \epsilon_y + \epsilon_z = \frac{\sigma_x + \sigma_y + \sigma_z - 2\nu(\sigma_x + \sigma_y + \sigma_z)}{E}$$

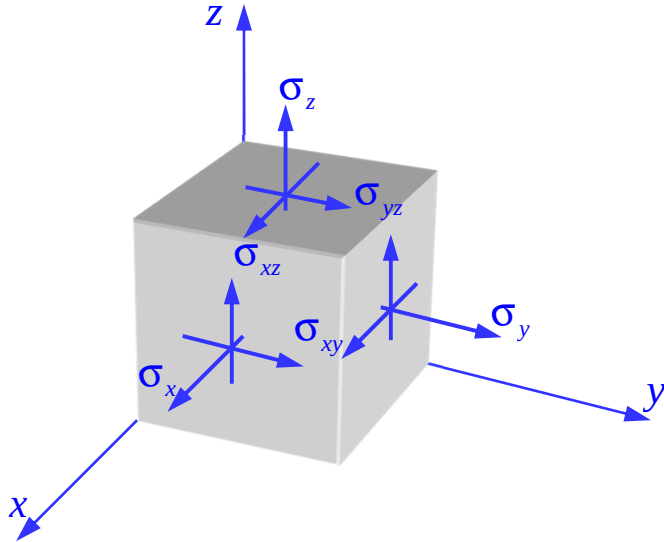
$$e = \frac{1-2\nu}{E} I_1$$

El tensor octaédrico de tensión es el responsable del cambio de volumen

$$\sigma = \frac{I_1}{3} \begin{pmatrix} 1 & 0 & 0 \\ 0 & 1 & 0 \\ 0 & 0 & 1 \end{pmatrix}$$



Ley de comportamiento en coordenadas cualesquiera



$$\epsilon_x = \frac{\sigma_x - \nu(\sigma_y + \sigma_z)}{E} \quad (1)$$

$$\epsilon_y = \frac{\sigma_y - \nu(\sigma_x + \sigma_z)}{E} \quad (2)$$

$$\epsilon_z = \frac{\sigma_z - \nu(\sigma_x + \sigma_y)}{E} \quad (3)$$

$$\epsilon_{xy} = \frac{(1+\nu)}{E} \sigma_{xy} \quad (4)$$

$$\epsilon_{xz} = \frac{(1+\nu)}{E} \sigma_{xz} \quad (5)$$

$$\epsilon_{yz} = \frac{(1+\nu)}{E} \sigma_{yz} \quad (7)$$

(1)+(2)+(3)

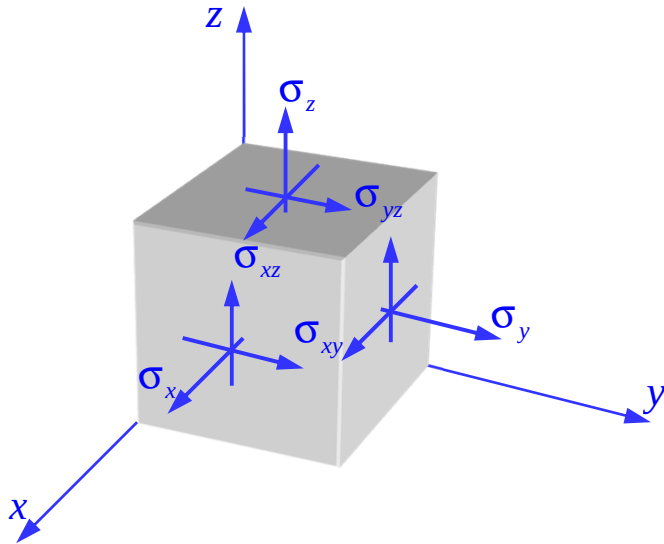
$$\epsilon_x + \epsilon_y + \epsilon_z = \frac{\sigma_x + \sigma_y + \sigma_z - 2\nu(\sigma_x + \sigma_y + \sigma_z)}{E} \quad \Rightarrow \quad e = \frac{1-2\nu}{E} I_1$$

$$\sigma = \begin{pmatrix} -P & 0 & 0 \\ 0 & -P & 0 \\ 0 & 0 & -P \end{pmatrix}$$

$$\frac{\Delta V}{V} = -3 \frac{1-2\nu}{E} P \quad \Rightarrow \quad P = -\frac{E}{3(1-2\nu)} \frac{\Delta V}{V}$$



Ley de comportamiento en coordenadas cualesquiera



$$\epsilon_x = \frac{\sigma_x - \nu(\sigma_y + \sigma_z)}{E} \quad (1)$$

$$\epsilon_y = \frac{\sigma_y - \nu(\sigma_x + \sigma_z)}{E} \quad (2)$$

$$\epsilon_z = \frac{\sigma_z - \nu(\sigma_x + \sigma_y)}{E} \quad (3)$$

$$\epsilon_{xy} = \frac{(1+\nu)}{E} \sigma_{xy} \quad (4)$$

$$\epsilon_{xz} = \frac{(1+\nu)}{E} \sigma_{xz} \quad (5)$$

$$\epsilon_{yz} = \frac{(1+\nu)}{E} \sigma_{yz} \quad (7)$$

(1)+(2)+(3)

$$\epsilon_x + \epsilon_y + \epsilon_z = \frac{\sigma_x + \sigma_y + \sigma_z - 2\nu(\sigma_x + \sigma_y + \sigma_z)}{E}$$

$$e = \frac{1-2\nu}{E} I_1$$

$$\sigma = \begin{pmatrix} -P & 0 & 0 \\ 0 & -P & 0 \\ 0 & 0 & -P \end{pmatrix}$$

Módulo
de rigidez
volumétrica

$$K = \frac{E}{3(1-2\nu)}$$

$$\frac{\Delta V}{V} = -3 \frac{1-2\nu}{E} P$$

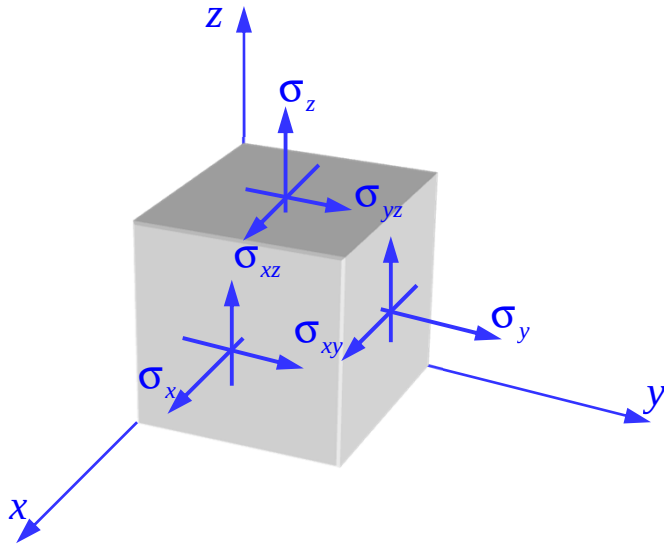
$$P = -\frac{E}{3(1-2\nu)} \frac{\Delta V}{V}$$

$$F = Ku$$



Mecánica de
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Ley de comportamiento en coordenadas cualesquiera



$$\epsilon_x = \frac{\sigma_x - \nu(\sigma_y + \sigma_z)}{E} \quad (1)$$

$$\epsilon_y = \frac{\sigma_y - \nu(\sigma_x + \sigma_z)}{E} \quad (2)$$

$$\epsilon_z = \frac{\sigma_z - \nu(\sigma_x + \sigma_y)}{E} \quad (3)$$

$$\epsilon_{xy} = \frac{(1+\nu)}{E} \sigma_{xy} \quad (4)$$

$$\epsilon_{xz} = \frac{(1+\nu)}{E} \sigma_{xz} \quad (5)$$

$$\epsilon_{yz} = \frac{(1+\nu)}{E} \sigma_{yz} \quad (7)$$

(1)+(2)+(3)

$$e = \frac{1-2\nu}{E} I_1$$

$$\sigma = \begin{pmatrix} -P & 0 & 0 \\ 0 & -P & 0 \\ 0 & 0 & -P \end{pmatrix}$$

$$P = -\frac{E}{3(1-2\nu)} \frac{\Delta V}{V}$$

Módulo de rigidez volumétrica

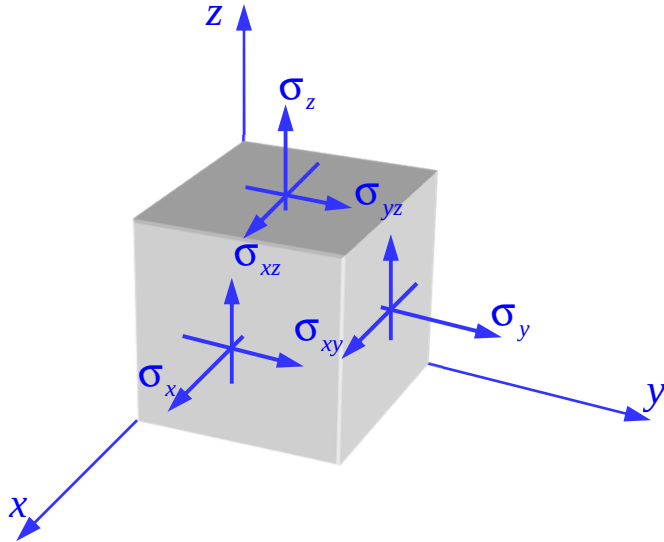
$$K = \frac{E}{3(1-2\nu)}$$

Nota: $K > 0 \iff 1-2\nu > 0 \iff 1/2 > \nu \iff 0 < \nu < 1/2$

$$\nu = 1/2 \implies K = \infty \implies \frac{\Delta V}{V} = -\frac{P}{K} = 0 \implies V \text{ cte}$$



Ley de comportamiento en coordenadas cualesquiera



$$\epsilon_x = \frac{\sigma_x - \nu(\sigma_y + \sigma_z)}{E} \quad (1)$$

$$\epsilon_y = \frac{\sigma_y - \nu(\sigma_x + \sigma_z)}{E} \quad (2)$$

$$\epsilon_z = \frac{\sigma_z - \nu(\sigma_x + \sigma_y)}{E} \quad (3)$$

$$\epsilon_{xy} = \frac{(1+\nu)}{E} \sigma_{xy} \quad (4)$$

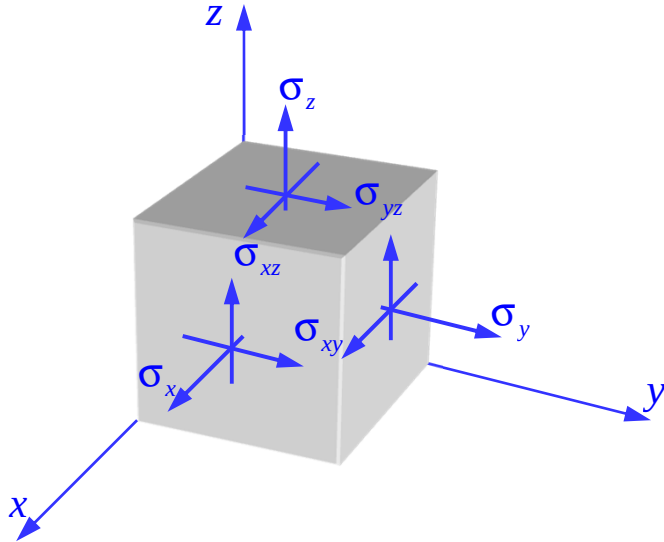
$$\epsilon_{xz} = \frac{(1+\nu)}{E} \sigma_{xz} \quad (5)$$

$$\epsilon_{yz} = \frac{(1+\nu)}{E} \sigma_{yz} \quad (7)$$

$$(1) \quad \epsilon_x = \frac{\sigma_x - \nu(\sigma_x + \sigma_y + \sigma_z) + \nu\sigma_x}{E} \quad \Rightarrow \quad \epsilon_x = \frac{1+\nu}{E} \sigma_x - \frac{\nu}{E} I_1$$



Ley de comportamiento en coordenadas cualesquiera



$$\epsilon_x = \frac{\sigma_x - \nu(\sigma_y + \sigma_z)}{E} \quad (1)$$

$$\epsilon_y = \frac{\sigma_y - \nu(\sigma_x + \sigma_z)}{E} \quad (2)$$

$$\epsilon_z = \frac{\sigma_z - \nu(\sigma_x + \sigma_y)}{E} \quad (3)$$

$$\epsilon_{xy} = \frac{(1+\nu)}{E} \sigma_{xy} \quad (4)$$

$$\epsilon_{xz} = \frac{(1+\nu)}{E} \sigma_{xz} \quad (5)$$

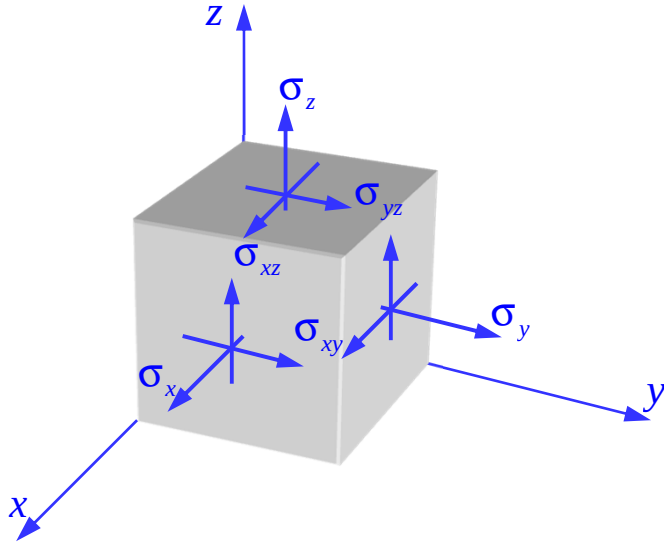
$$\epsilon_{yz} = \frac{(1+\nu)}{E} \sigma_{yz} \quad (7)$$

$$(1) \quad \epsilon_x = \frac{\sigma_x - \nu(\sigma_x + \sigma_y + \sigma_z) + \nu\sigma_x}{E} \quad \Rightarrow \quad \epsilon_x = \frac{1+\nu}{E} \sigma_x - \frac{\nu}{E} I_1 \quad \Rightarrow$$

$$\Rightarrow \quad \sigma_x = \left(\frac{E}{1+\nu} \epsilon_x + \frac{\nu E}{(1-2\nu)(1+\nu)} e \right)$$



Ley de comportamiento en coordenadas cualesquiera



$$\epsilon_x = \frac{\sigma_x - \nu(\sigma_y + \sigma_z)}{E} \quad (1)$$

$$\epsilon_y = \frac{\sigma_y - \nu(\sigma_x + \sigma_z)}{E} \quad (2)$$

$$\epsilon_z = \frac{\sigma_z - \nu(\sigma_x + \sigma_y)}{E} \quad (3)$$

$$\epsilon_{xy} = \frac{(1+\nu)}{E} \sigma_{xy} \quad (4)$$

$$\epsilon_{xz} = \frac{(1+\nu)}{E} \sigma_{xz} \quad (5)$$

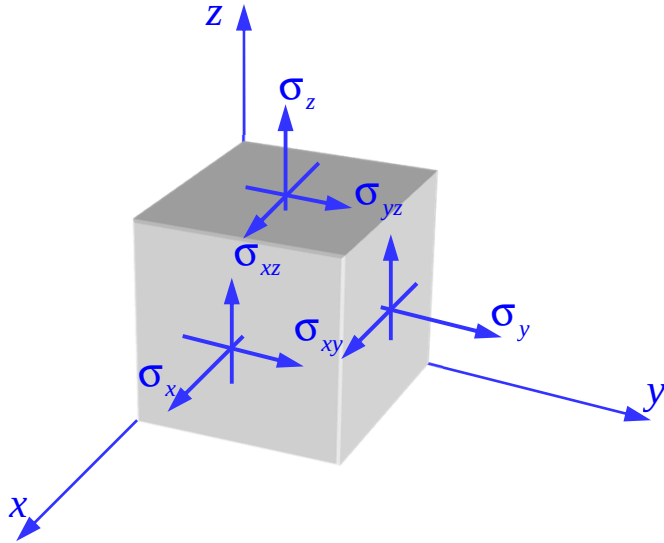
$$\epsilon_{yz} = \frac{(1+\nu)}{E} \sigma_{yz} \quad (7)$$

$$(1) \quad \epsilon_x = \frac{\sigma_x - \nu(\sigma_x + \sigma_y + \sigma_z) + \nu\sigma_x}{E} \quad \Rightarrow \quad \epsilon_x = \frac{1+\nu}{E} \sigma_x - \frac{\nu}{E} I_1 \quad \Rightarrow$$

$$\Rightarrow \quad \sigma_x = \underbrace{\left(\frac{E}{1+\nu} \right)}_{2G} \epsilon_x + \frac{\nu E}{(1-2\nu)(1+\nu)} e$$



Ley de comportamiento en coordenadas cualesquiera



$$\epsilon_x = \frac{\sigma_x - \nu(\sigma_y + \sigma_z)}{E} \quad (1)$$

$$\epsilon_y = \frac{\sigma_y - \nu(\sigma_x + \sigma_z)}{E} \quad (2)$$

$$\epsilon_z = \frac{\sigma_z - \nu(\sigma_x + \sigma_y)}{E} \quad (3)$$

$$\epsilon_{xy} = \frac{(1+\nu)}{E} \sigma_{xy} \quad (4)$$

$$\epsilon_{xz} = \frac{(1+\nu)}{E} \sigma_{xz} \quad (5)$$

$$\epsilon_{yz} = \frac{(1+\nu)}{E} \sigma_{yz} \quad (7)$$

$$(1) \quad \epsilon_x = \frac{\sigma_x - \nu(\sigma_x + \sigma_y + \sigma_z) + \nu\sigma_x}{E} \quad \Rightarrow \quad \epsilon_x = \frac{1+\nu}{E} \sigma_x - \frac{\nu}{E} I_1 \quad \Rightarrow$$

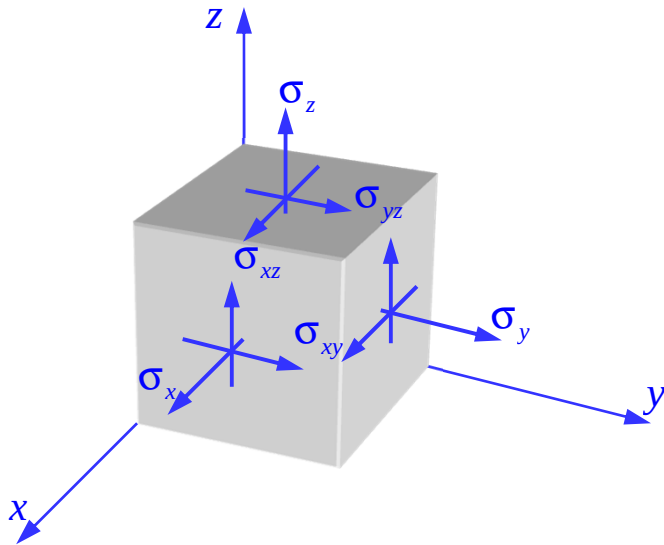
$$\Rightarrow \quad \sigma_x = \left(\underbrace{\frac{E}{1+\nu}}_{2G} \epsilon_x + \underbrace{\frac{\nu E}{(1-2\nu)(1+\nu)}}_{\lambda} e \right)$$

Constante de Lamé

Nota: $\lambda > 0$



Ley de comportamiento en coordenadas cualesquiera



$$\epsilon_x = \frac{\sigma_x - \nu(\sigma_y + \sigma_z)}{E} \quad (1)$$

$$\epsilon_y = \frac{\sigma_y - \nu(\sigma_x + \sigma_z)}{E} \quad (2)$$

$$\epsilon_z = \frac{\sigma_z - \nu(\sigma_x + \sigma_y)}{E} \quad (3)$$

Ecs. de Hooke

$$\epsilon_{xy} = \frac{(1+\nu)}{E} \sigma_{xy} \quad (4)$$

$$\epsilon_{xz} = \frac{(1+\nu)}{E} \sigma_{xz} \quad (5)$$

$$\epsilon_{yz} = \frac{(1+\nu)}{E} \sigma_{yz} \quad (7)$$

$$\sigma_x = 2G\epsilon_x + \lambda e \quad (8)$$

$$\sigma_y = 2G\epsilon_y + \lambda e \quad (9)$$

$$\sigma_z = 2G\epsilon_z + \lambda e \quad (10)$$

Ecs. de Lamé

$$\sigma_{xy} = 2G\epsilon_{xy} \quad (11)$$

$$\sigma_{xz} = 2G\epsilon_{xz} \quad (12)$$

$$\sigma_{yz} = 2G\epsilon_{yz} \quad (13)$$



EL PROBLEMA ELÁSTICO

$F(x,y,z)$

$u(x,y,z)$

Equilibrio

Ecs. compatibilidad

$\sigma(x,y,z)$

Comportamiento

$\epsilon(x,y,z)$

$$\frac{\partial \sigma_{xx}}{\partial x} + \frac{\partial \tau_{yx}}{\partial y} + \frac{\partial \tau_{zx}}{\partial z} + X = 0$$

$$\frac{\partial \tau_{xy}}{\partial x} + \frac{\partial \sigma_{yy}}{\partial y} + \frac{\partial \tau_{zy}}{\partial z} + Y = 0$$

$$\frac{\partial \tau_{xz}}{\partial x} + \frac{\partial \tau_{yz}}{\partial y} + \frac{\partial \sigma_{zz}}{\partial z} + Z = 0$$

$$\epsilon_x = \frac{\sigma_x - \nu(\sigma_y + \sigma_z)}{E} \quad \epsilon_{xy} = \frac{(1+\nu)}{E} \sigma_{xy}$$

$$\epsilon_y = \frac{\sigma_y - \nu(\sigma_x + \sigma_z)}{E} \quad \epsilon_{xz} = \frac{(1+\nu)}{E} \sigma_{xz}$$

$$\epsilon_z = \frac{\sigma_z - \nu(\sigma_x + \sigma_y)}{E} \quad \epsilon_{yz} = \frac{(1+\nu)}{E} \sigma_{yz}$$

$$\frac{\partial^2 \epsilon_{xy}}{\partial x \partial y} = \frac{1}{2} \left(\frac{\partial^2 \epsilon_x}{\partial y^2} + \frac{\partial^2 \epsilon_y}{\partial x^2} \right)$$

$$\frac{\partial^2 \epsilon_{xz}}{\partial x \partial z} = \frac{1}{2} \left(\frac{\partial^2 \epsilon_x}{\partial z^2} + \frac{\partial^2 \epsilon_z}{\partial x^2} \right)$$

$$\frac{\partial^2 \epsilon_{yz}}{\partial y \partial z} = \frac{1}{2} \left(\frac{\partial^2 \epsilon_y}{\partial z^2} + \frac{\partial^2 \epsilon_z}{\partial y^2} \right)$$

$$\frac{\partial^2 \epsilon_x}{\partial y \partial z} = \frac{\partial}{\partial x} \left(\frac{\partial \epsilon_{xz}}{\partial y} + \frac{\partial \epsilon_{xy}}{\partial z} - \frac{\partial \epsilon_{yz}}{\partial x} \right)$$

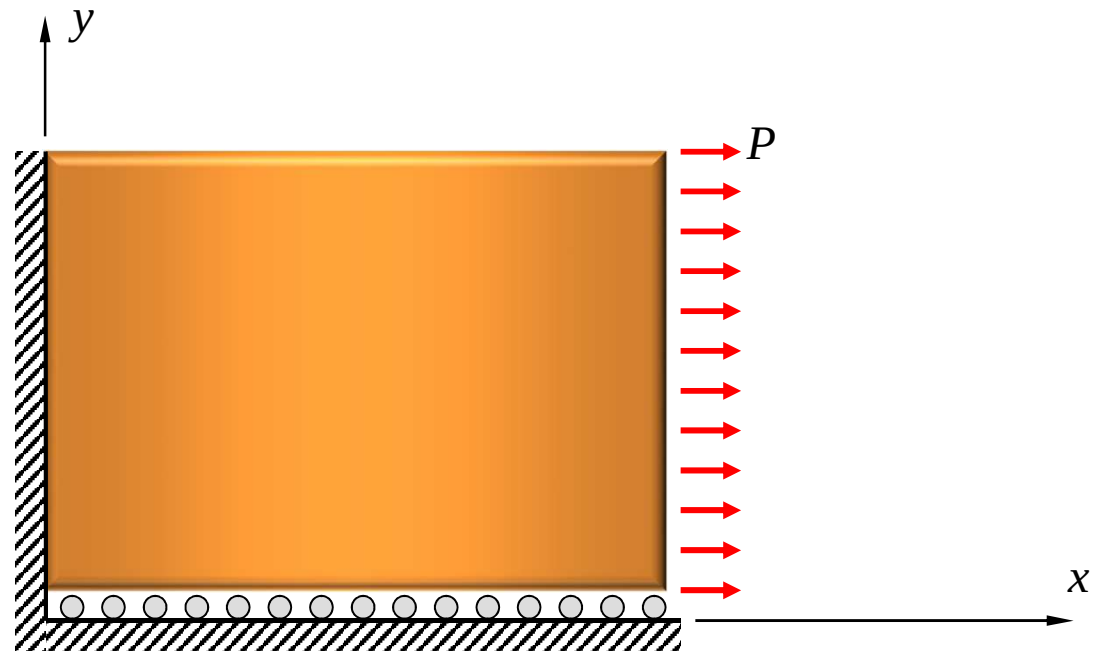
$$\frac{\partial^2 \epsilon_y}{\partial z \partial x} = \frac{\partial}{\partial y} \left(\frac{\partial \epsilon_{yx}}{\partial z} + \frac{\partial \epsilon_{yz}}{\partial x} - \frac{\partial \epsilon_{zx}}{\partial y} \right)$$

$$\frac{\partial^2 \epsilon_z}{\partial x \partial y} = \frac{\partial}{\partial z} \left(\frac{\partial \epsilon_{zx}}{\partial y} + \frac{\partial \epsilon_{zy}}{\partial x} - \frac{\partial \epsilon_{xy}}{\partial z} \right)$$



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Condiciones de contorno

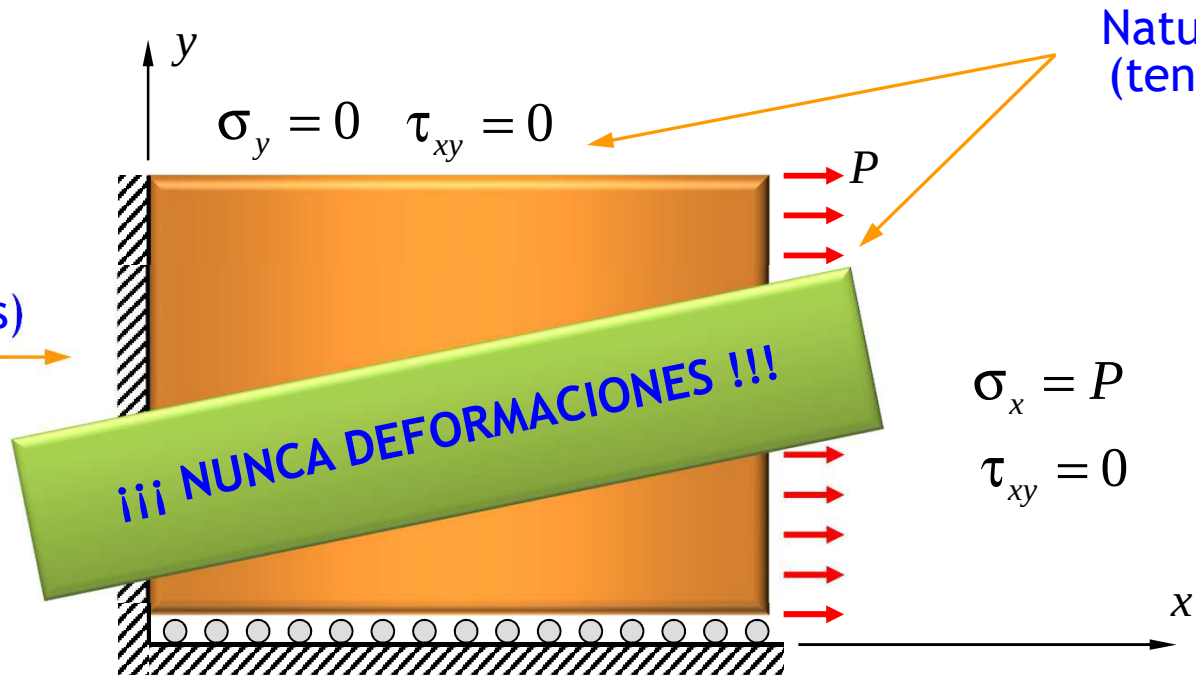


Condiciones de contorno

Esenciales
(desplazamientos)

$$u_x = 0$$

$$u_y = 0$$



Naturales
(tensión)

$$\sigma_y = 0 \quad \tau_{xy} = 0$$

P

$$\sigma_x = P$$

$$\tau_{xy} = 0$$

x

iii NUNCA DEFORMACIONES !!!

Mixtas

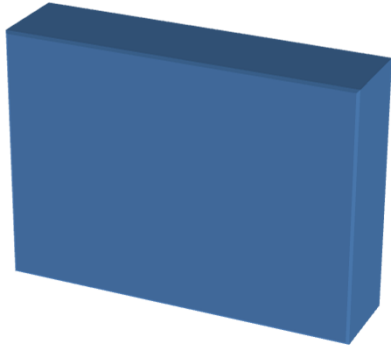
$$\tau_{xy} = 0$$

$$u_y = 0$$

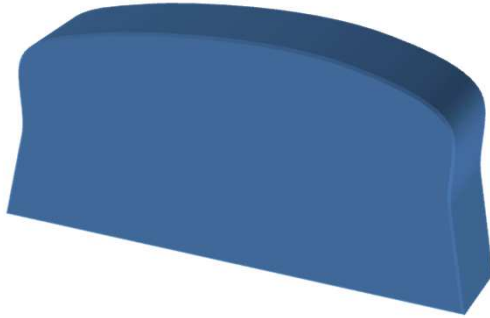


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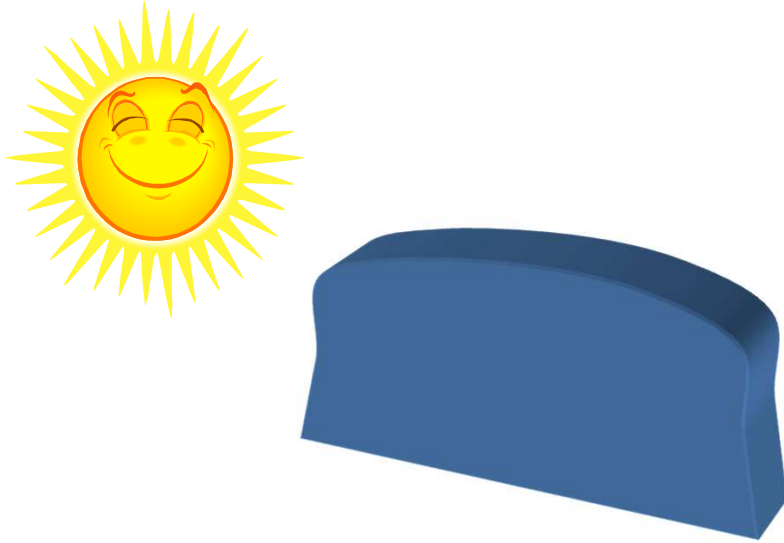
El problema térmico. Una primera aproximación



El problema térmico. Una primera aproximación



El problema térmico. Una primera aproximación



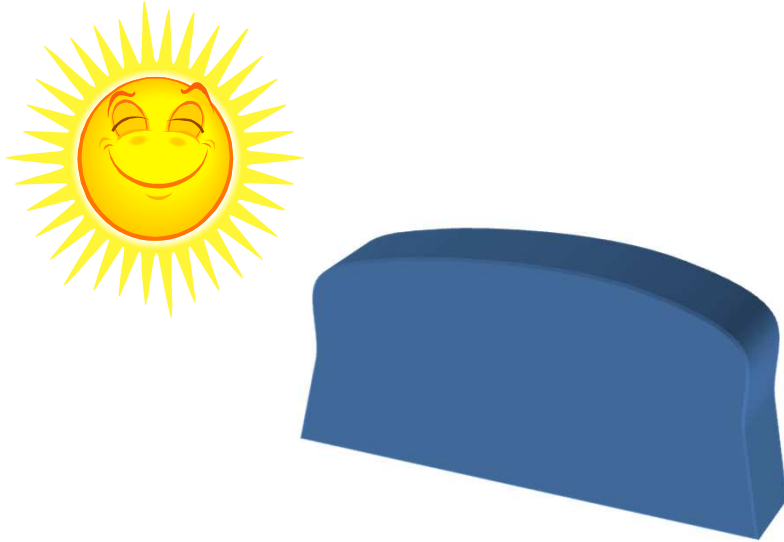
Si $\alpha \neq f(T)$

$$\varepsilon = \frac{\Delta l}{l_0} = \frac{\alpha l_0 \Delta T}{l_0}$$



$$\left\{ \begin{array}{l} \varepsilon_x = \alpha \Delta T \\ \varepsilon_y = \alpha \Delta T \\ \varepsilon_z = \alpha \Delta T \end{array} \right.$$

El problema térmico. Una primera aproximación



Si $\alpha \neq f(T)$

$$\varepsilon = \frac{\Delta l}{l_0} = \frac{\alpha l_0 \Delta T}{l_0} \Rightarrow \begin{cases} \varepsilon_x = \alpha \Delta T \\ \varepsilon_y = \alpha \Delta T \\ \varepsilon_z = \alpha \Delta T \end{cases}$$

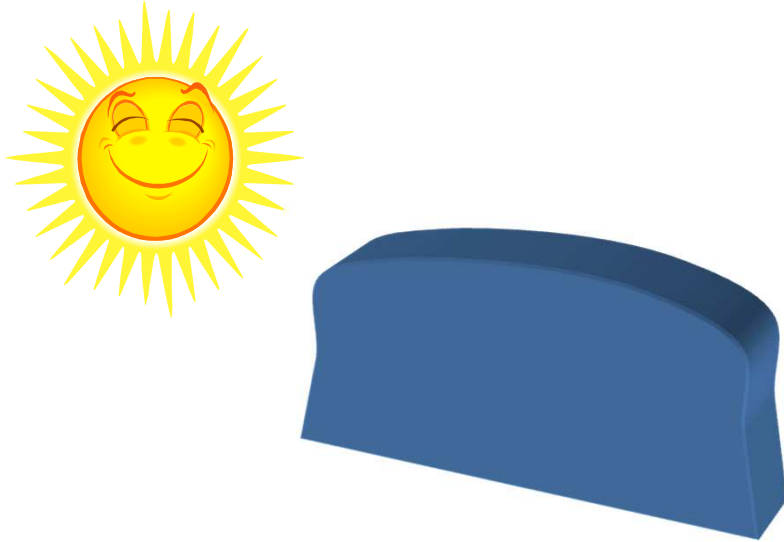
Si no hay restricciones al movimiento

$$\varepsilon = \begin{pmatrix} \alpha \Delta T & 0 & 0 \\ 0 & \alpha \Delta T & 0 \\ 0 & 0 & \alpha \Delta T \end{pmatrix}$$

$$\sigma = \begin{pmatrix} 0 & 0 & 0 \\ 0 & 0 & 0 \\ 0 & 0 & 0 \end{pmatrix}$$



El problema térmico. Una primera aproximación



Si $\alpha \neq f(T)$

$$\varepsilon = \frac{\Delta l}{l_0} = \frac{\alpha l_0 \Delta T}{l_0} \Rightarrow \begin{cases} \varepsilon_x = \alpha \Delta T \\ \varepsilon_y = \alpha \Delta T \\ \varepsilon_z = \alpha \Delta T \end{cases}$$

Si no hay restricciones al movimiento

$$\varepsilon = \begin{pmatrix} \alpha \Delta T & 0 & 0 \\ 0 & \alpha \Delta T & 0 \\ 0 & 0 & \alpha \Delta T \end{pmatrix}$$

$$\sigma = \begin{pmatrix} 0 & 0 & 0 \\ 0 & 0 & 0 \\ 0 & 0 & 0 \end{pmatrix}$$

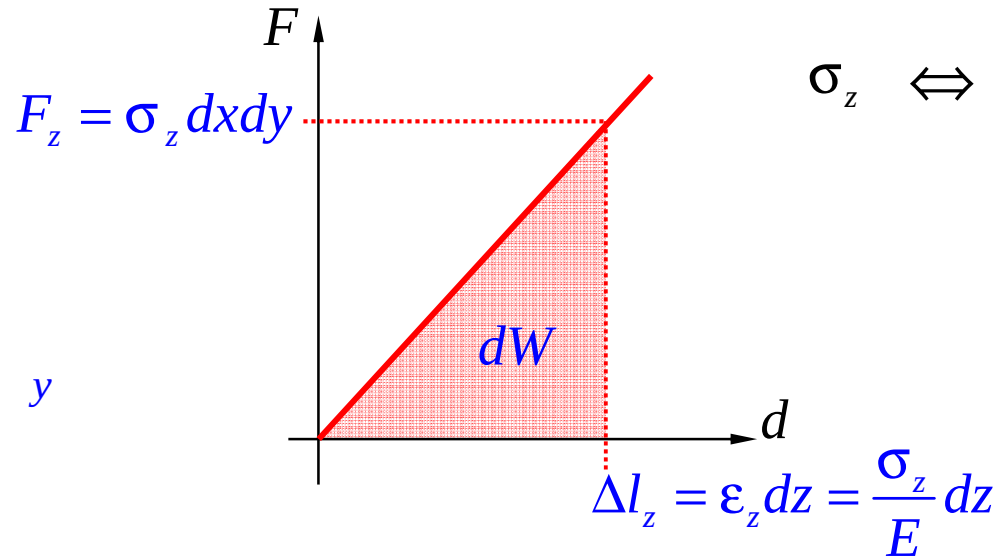
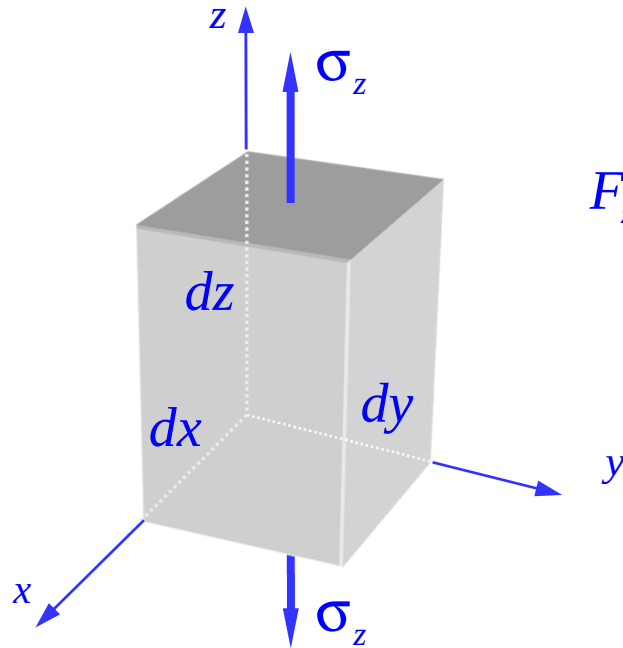
$$T = T(x, y, z)$$



$$\varepsilon = \varepsilon^{mec} + \varepsilon^{ter}$$



Energía de deformación



$$\epsilon_z = \sigma_z / E \Leftrightarrow \Delta l_z = \epsilon_z dz$$

$$\sigma_z \Leftrightarrow F_z = \sigma_z dx dy$$

$$dW = \frac{1}{2} F \Delta l_z = \frac{1}{2} \sigma_z \epsilon_z dx dy dz = \frac{1}{2} \sigma_z \epsilon_z dV$$

Energía unitaria de
deformación

Energía de
deformación
por unidad de volumen

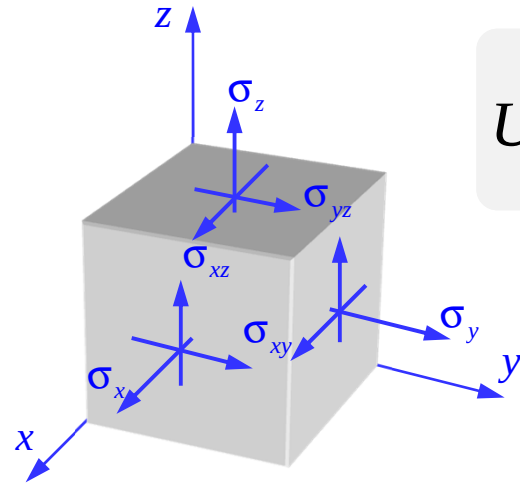
Energía de deformación

$$\frac{dW}{dV} = \frac{1}{2} \sigma_z \epsilon_z := U$$



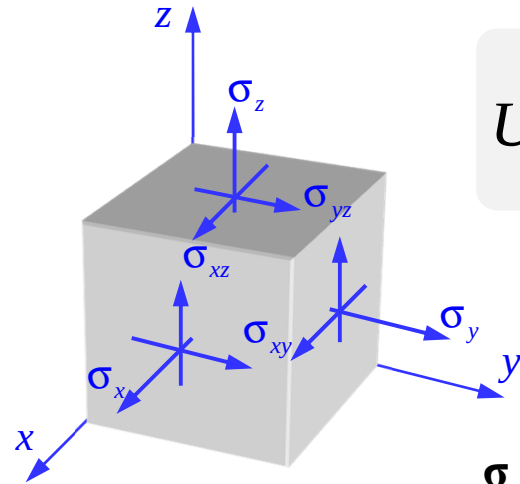
Mecánica de
Medios Continuos y
Teoría de Estructuras

Energía de deformación



$$U = \frac{1}{2} \left(\sigma_x \varepsilon_x + \sigma_y \varepsilon_y + \sigma_z \varepsilon_z + \tau_{xy} \gamma_{xy} + \tau_{xz} \gamma_{xz} + \tau_{yz} \gamma_{yz} \right)$$

Energía de deformación



$$U = \frac{1}{2} \left(\sigma_x \varepsilon_x + \sigma_y \varepsilon_y + \sigma_z \varepsilon_z + \tau_{xy} \gamma_{xy} + \tau_{xz} \gamma_{xz} + \tau_{yz} \gamma_{yz} \right)$$

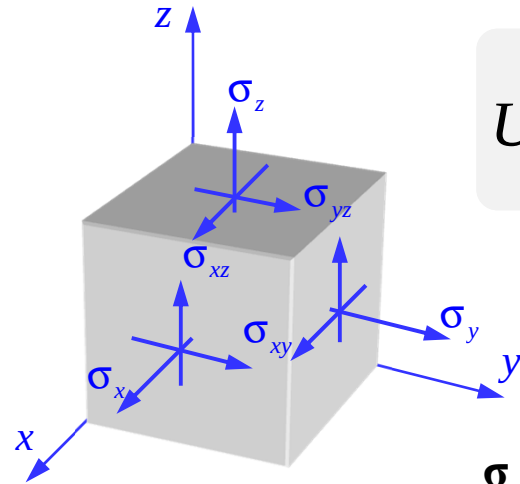
$$\left. \begin{aligned} \boldsymbol{\sigma} &= \frac{I_1}{3} \begin{pmatrix} 1 & 0 & 0 \\ 0 & 1 & 0 \\ 0 & 0 & 1 \end{pmatrix} \\ \boldsymbol{\varepsilon} &= \frac{e}{3} \begin{pmatrix} 1 & 0 & 0 \\ 0 & 1 & 0 \\ 0 & 0 & 1 \end{pmatrix} \end{aligned} \right\}$$



$$U_0 = \frac{eI_1}{6}$$

Energía
de cambio
de volumen

Energía de deformación



$$U = \frac{1}{2} \left(\sigma_x \varepsilon_x + \sigma_y \varepsilon_y + \sigma_z \varepsilon_z + \tau_{xy} \gamma_{xy} + \tau_{xz} \gamma_{xz} + \tau_{yz} \gamma_{yz} \right)$$

$$\left. \begin{aligned} \boldsymbol{\sigma} &= \frac{I_1}{3} \begin{pmatrix} 1 & 0 & 0 \\ 0 & 1 & 0 \\ 0 & 0 & 1 \end{pmatrix} \\ \boldsymbol{\varepsilon} &= \frac{e}{3} \begin{pmatrix} 1 & 0 & 0 \\ 0 & 1 & 0 \\ 0 & 0 & 1 \end{pmatrix} \end{aligned} \right\}$$



$$U_0 = \frac{eI_1}{6}$$

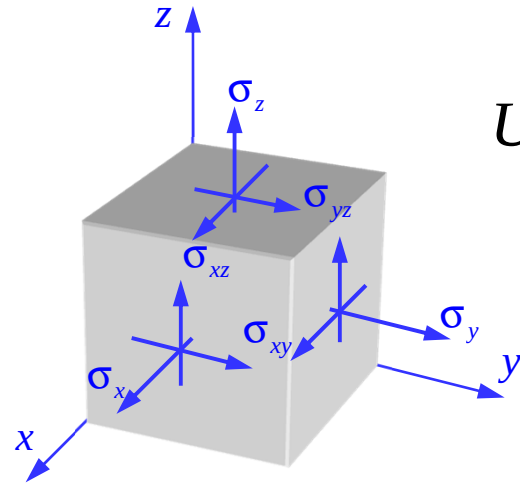
Energía
de cambio
de volumen

Energía de cambio de forma

$$U_d = U - U_0 = \frac{1+\nu}{6E} \left[(\sigma_x - \sigma_y)^2 + (\sigma_x - \sigma_z)^2 + (\sigma_y - \sigma_z)^2 \right] + \frac{1}{2G} (\tau_{xy}^2 + \tau_{xz}^2 + \tau_{yz}^2)$$



Energía de deformación

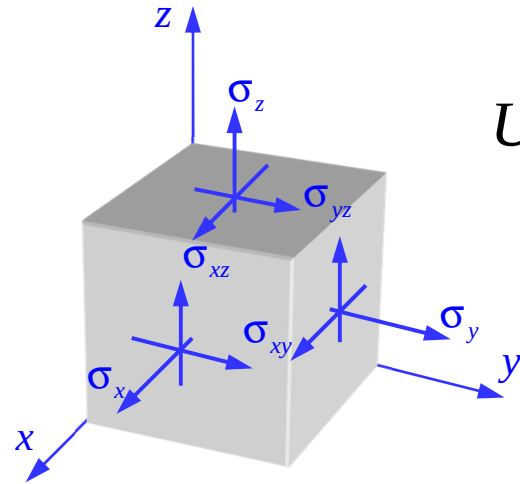


$$U = \frac{1}{2} \left(\sigma_x \varepsilon_x + \sigma_y \varepsilon_y + \sigma_z \varepsilon_z + \tau_{xy} \gamma_{xy} + \tau_{xz} \gamma_{xz} + \tau_{yz} \gamma_{yz} \right)$$

En ausencia de
fenómenos disipativos

Energía potencial

Energía de deformación



$$U = \frac{1}{2} \left(\sigma_x \varepsilon_x + \sigma_y \varepsilon_y + \sigma_z \varepsilon_z + \tau_{xy} \gamma_{xy} + \tau_{xz} \gamma_{xz} + \tau_{yz} \gamma_{yz} \right)$$

En ausencia de
fenómenos disipativos

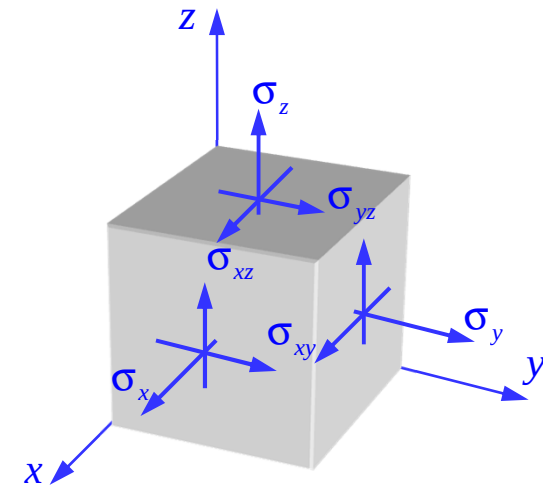
Energía potencial

Energía potencial total

$$E_p = \frac{1}{2} \int \left(\sigma_x \varepsilon_x + \sigma_y \varepsilon_y + \sigma_z \varepsilon_z + \tau_{xy} \gamma_{xy} + \tau_{xz} \gamma_{xz} + \tau_{yz} \gamma_{yz} \right) dV$$

Igual a la energía total de deformación e igual al trabajo externo





Tensión plana vs. Deformación plana

Ecs. de Hooke

$$\epsilon_x = \frac{\sigma_x - \nu(\sigma_y + \sigma_z)}{E} \quad \epsilon_y = \frac{\sigma_y - \nu(\sigma_x + \sigma_z)}{E} \quad \epsilon_z = \frac{\sigma_z - \nu(\sigma_x + \sigma_y)}{E}$$

$$\epsilon_{xy} = \frac{(1+\nu)}{E} \sigma_{xy} \quad \epsilon_{xz} = \frac{(1+\nu)}{E} \sigma_{xz} \quad \epsilon_{yz} = \frac{(1+\nu)}{E} \sigma_{yz}$$

Tensión
Plana

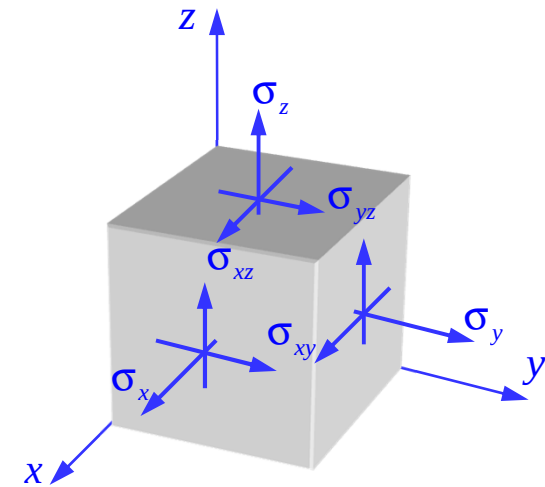
$$\boldsymbol{\sigma} = \begin{pmatrix} \sigma_x & \sigma_{xy} & 0 \\ \sigma_{xy} & \sigma_y & 0 \\ 0 & 0 & 0 \end{pmatrix}$$



$$\boldsymbol{\epsilon} = \begin{pmatrix} \epsilon_x & \epsilon_{xy} & 0 \\ \epsilon_{xy} & \epsilon_y & 0 \\ 0 & 0 & -\nu(\sigma_x + \sigma_y)/E \end{pmatrix}$$

Deformación
Plana

$$\Leftrightarrow \sigma_x = -\sigma_y$$



Tensión Plana

$$\boldsymbol{\sigma} = \begin{pmatrix} \sigma_x & \sigma_{xy} & 0 \\ \sigma_{xy} & \sigma_y & 0 \\ 0 & 0 & 0 \end{pmatrix}$$

Ecs. de Hooke

$$\epsilon_x = \frac{\sigma_x - \nu(\sigma_y + \sigma_z)}{E}$$

$$\epsilon_y = \frac{\sigma_y - \nu(\sigma_x + \sigma_z)}{E}$$

$$\epsilon_{xy} = \frac{(1+\nu)}{E} \sigma_{xy}$$

$$\epsilon_{xz} = \frac{(1+\nu)}{E} \sigma_{xz}$$

$$\epsilon_z = \frac{\sigma_z - \nu(\sigma_x + \sigma_y)}{E}$$

$$\epsilon_{yz} = \frac{(1+\nu)}{E} \sigma_{yz}$$

$$\boldsymbol{\epsilon} = \begin{pmatrix} \epsilon_x & \epsilon_{xy} & 0 \\ \epsilon_{xy} & \epsilon_y & 0 \\ 0 & 0 & -\nu(\sigma_x + \sigma_y) / E \end{pmatrix}$$

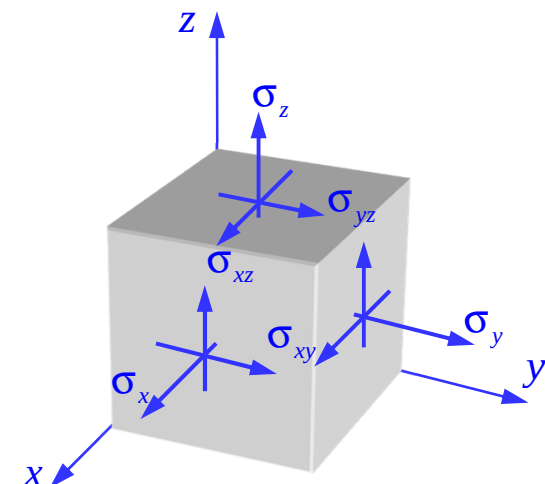
Deformación Plana

$$\Leftrightarrow \sigma_x = -\sigma_y$$

Tensión plana vs. Deformación plana



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Ecs. de Hooke

$$\varepsilon_x = \frac{\sigma_x - \nu(\sigma_y + \sigma_z)}{E}$$

$$\varepsilon_y = \frac{\sigma_y - \nu(\sigma_x + \sigma_z)}{E}$$

$$\varepsilon_z = \frac{\sigma_z - \nu(\sigma_x + \sigma_y)}{E}$$

$$\varepsilon_{xy} = \frac{(1+\nu)}{E} \sigma_{xy}$$

$$\varepsilon_{xz} = \frac{(1+\nu)}{E} \sigma_{xz}$$

$$\varepsilon_{yz} = \frac{(1+\nu)}{E} \sigma_{yz}$$

Tensión Plana

$$\boldsymbol{\sigma} = \begin{pmatrix} \sigma_x & \sigma_{xy} & 0 \\ \sigma_{xy} & \sigma_y & 0 \\ 0 & 0 & 0 \end{pmatrix}$$

$$\boldsymbol{\varepsilon} = \begin{pmatrix} \varepsilon_x & \varepsilon_{xy} & 0 \\ \varepsilon_{xy} & \varepsilon_y & 0 \\ 0 & 0 & -\nu(\sigma_x + \sigma_y)/E \end{pmatrix}$$

Deformación Plana

$$\Leftrightarrow \sigma_x = -\sigma_y$$

Deformación Plana

$$\boldsymbol{\varepsilon} = \begin{pmatrix} \varepsilon_x & \varepsilon_{xy} & 0 \\ \varepsilon_{xy} & \varepsilon_y & 0 \\ 0 & 0 & 0 \end{pmatrix}$$

$$\boldsymbol{\sigma} = \begin{pmatrix} \sigma_x & \sigma_{xy} & 0 \\ \sigma_{xy} & \sigma_y & 0 \\ 0 & 0 & \lambda(\varepsilon_x + \varepsilon_y) \end{pmatrix}$$

Tensión Plana

$$\Leftrightarrow \varepsilon_x = -\varepsilon_y$$

Ecs. de Lamé

$$\sigma_x = 2G\varepsilon_x + \lambda e \quad \sigma_y = 2G\varepsilon_y + \lambda e \quad \sigma_z = 2G\varepsilon_z + \lambda e$$

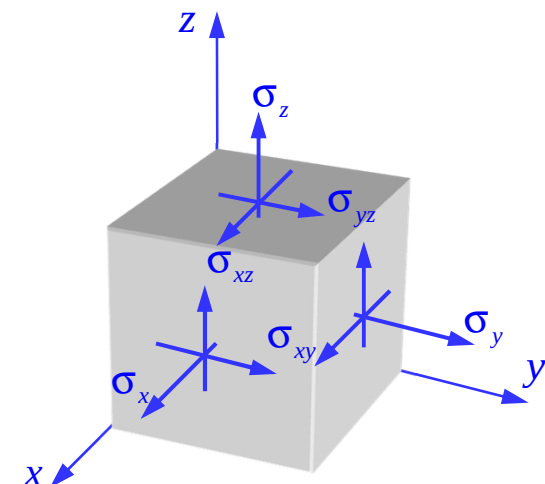
$$\sigma_{xy} = 2G\varepsilon_{xy}$$

$$\sigma_{xz} = 2G\varepsilon_{xz}$$

$$\sigma_{yz} = 2G\varepsilon_{yz}$$



Mecánica de Medios Continuos y Teoría de Estructuras



Ecs. de Hooke

$$\varepsilon_x = \frac{\sigma_x - \nu(\sigma_y + \sigma_z)}{E}$$

$$\varepsilon_y = \frac{\sigma_y - \nu(\sigma_x + \sigma_z)}{E}$$

$$\varepsilon_z = \frac{\sigma_z - \nu(\sigma_x + \sigma_y)}{E}$$

$$\varepsilon_{xy} = \frac{(1+\nu)}{E} \sigma_{xy}$$

$$\varepsilon_{xz} = \frac{(1+\nu)}{E} \sigma_{xz}$$

$$\varepsilon_{yz} = \frac{(1+\nu)}{E} \sigma_{yz}$$

Tensión Plana

$$\boldsymbol{\sigma} = \begin{pmatrix} \sigma_x & \sigma_{xy} & 0 \\ \sigma_{xy} & \sigma_y & 0 \\ 0 & 0 & 0 \end{pmatrix}$$

$$\boldsymbol{\varepsilon} = \begin{pmatrix} \varepsilon_x & \varepsilon_{xy} & 0 \\ \varepsilon_{xy} & \varepsilon_y & 0 \\ 0 & 0 & -\nu(\sigma_x + \sigma_y)/E \end{pmatrix}$$

Deformación Plana

$$\Leftrightarrow \sigma_x = -\sigma_y$$

Deformación Plana

$$\boldsymbol{\varepsilon} = \begin{pmatrix} \varepsilon_x & \varepsilon_{xy} & 0 \\ \varepsilon_{xy} & \varepsilon_y & 0 \\ 0 & 0 & 0 \end{pmatrix}$$

$$\boldsymbol{\sigma} = \begin{pmatrix} \sigma_x & \sigma_{xy} & 0 \\ \sigma_{xy} & \sigma_y & 0 \\ 0 & 0 & \lambda(\varepsilon_x + \varepsilon_y) \end{pmatrix}$$

Tensión Plana

$$\Leftrightarrow \varepsilon_x = -\varepsilon_y$$

Ecs. de Lamé

$$\sigma_x = 2G\varepsilon_x + \lambda e \quad \sigma_y = 2G\varepsilon_y + \lambda e \quad \sigma_z = 2G\varepsilon_z + \lambda e$$

$$\sigma_{xy} = 2G\varepsilon_{xy}$$

$$\sigma_{xz} = 2G\varepsilon_{xz}$$

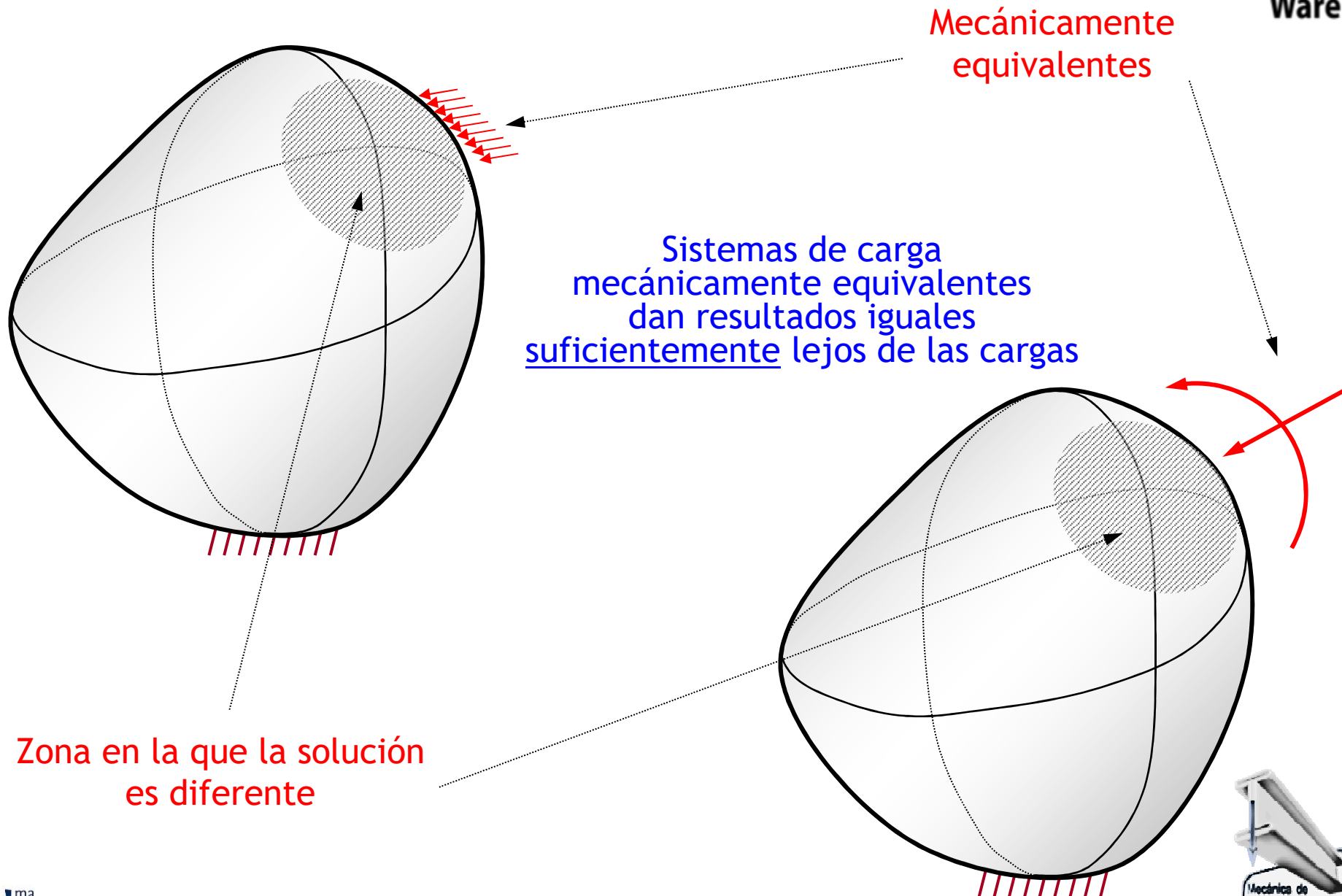
$$\sigma_{yz} = 2G\varepsilon_{yz}$$



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Idéntica solución

